



Department
for Education

Managing Older Buildings:

A guide for estates maintenance staff

July 2026

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Summary

This publication from the Department for Education (DfE) provides non-statutory guidance on the types of construction commonly found in the school estate. It has been produced to help building users, owners and maintenance staff to identify building systems and help prioritise future maintenance and repair of their buildings.

Who is this publication for?

This guidance is for:

- local authorities (for community, voluntary-controlled schools, foundation schools and maintained nursery schools)
- academy trusts (for academies and free schools)
- governing bodies (for voluntary-aided schools)
- school/college leaders, staff and governing bodies in; further education colleges and designated institutions, sixth form colleges, maintained schools, academies and free schools, pupil referral units, City Technology Colleges, non-maintained special schools and maintained nursery schools
- technical professionals involved in the design, construction and maintenance of school premises.

Academy trusts with a religious character and the responsible bodies for voluntary-aided and voluntary controlled schools should engage with the land/site trustees for their land and buildings and the relevant religious body, most often their Diocese.

Revision history

This table lists the key changes in each update.

ISO revision code	ISO status code	Date	Amendment
C01	A	2024-04-25	First issue of guidance
C02	A	2025-12-12	Clinker and High Alumina Cement forms of concrete added to Appendix B (common materials). New Appendix C (common components) added
C03	A	2026-07-16	SCOLA added to Appendix A. TonkinDek added to Appendix C. Content for concrete, clinker concrete, steelwork and masonry updated in Appendix B. Content for proprietary timber 'ply web' systems updated in Appendix C. New Appendix D (historic design practices) added

Introduction

All buildings need to be maintained and managed effectively to remain comfortable and safe. It is important that building owners, users and maintenance staff have knowledge of the construction types and materials used in their buildings.

Holding historical records such as construction drawings and records of alterations, photographs, regular condition surveys and structural assessments will help building owners to understand their buildings. This knowledge will allow them to plan appropriate maintenance and monitor the building fabric for any early signs of deterioration. With effective maintenance most buildings continue to perform long after their intended design life.

This guidance provides information relating to the typical construction types used in the school estate, particularly those commonly used in the period between 1945 and the mid-1970s when a significant number of school buildings were constructed.

The guidance predominantly focuses on common maintenance items for load bearing structural framing and some cladding elements. For guidance concerning the day-to-day maintenance of finishes and decoration, and the maintenance of other components in the building (e.g., heating and electrical systems), please refer to the [Good Estate Management for Schools](#).

The guidance is split into two parts with four appendices.

Part 1 provides a brief history of school building in England.

Part 2 provides information about the types of construction that are typically used in the school estate. It should be read in conjunction with Appendices A, B, C and D.

Appendix A provides information about four types of ‘system build’ that were common in the post-war period. These forms of construction require special mention as they used evolving construction methods. Therefore, maintenance plans should consider the system as a whole as well as its constituent parts and their materials.

Appendix B provides information about the construction materials commonly used in the construction types outlined in Part 2 and the systems in Appendix A. It gives further details on the common signs of deterioration, how these should be treated in the short term and where to seek further advice.

Appendix C provides information about the proprietary construction components commonly used in the construction types outlined in Part 2. It notes identifying features and common maintenance issues to consider that may require further assessment.

Appendix D provides information about historic building design practices which should be considered when planning regular maintenance or more extensive modification to buildings.

Additional considerations

Scope of guidance

This guidance is intended to give those responsible for planning and carrying out maintenance a high-level overview of the types of construction that may be present in their estates and to help identify these common types of construction. It outlines some of the common areas that should be considered to ensure buildings remain serviceable and structural elements do not deteriorate.

It is not intended to be exhaustive, if the reader requires further information, they should consult a suitably qualified building professional who will be able to provide more detailed information which is tailored to their specific buildings, taking into account wider condition issues.

Managing structural issues

A structural issue is a significant adverse change in the condition of a building's structure (including footings, foundations, structural portions of load bearing walls, structural floors and subfloors, primary roof structure, lift shafts, and structural columns and beams) that may cause harm or full or partial closure of a building. Further information and guidance can be found on [Manage Your Education Estate \(MYEE\)](#). You can also raise an enquiry using the [Customer Help Portal](#) which will be directed to officials in the Education Estates Directorate.

Asbestos

Your school buildings may contain asbestos if any part was built before the year 2000. It is extremely important that any asbestos present in your estate is located and managed effectively.

Before carrying out any investigative work other than visual inspection, you must consult the [DfE's guidance on asbestos](#). It should be used alongside more detailed advice, such as the Health and Safety Executive's (HSE) [duty to manage asbestos in buildings](#) and [asbestos management checklist for schools](#).

Following the steps set out in the above guidance should protect the health of staff, pupils, students, contractors and visitors to the estate from the risk of exposure to asbestos.

Health & Safety considerations

As well as asbestos, wider health and safety considerations should be considered when planning building maintenance. These include the safety of those carrying out the works as well as all other building users.

Building maintenance should be carried out by suitably qualified professionals and only when planned risk assessments and method statements, appropriate to the works, are in place. Depending on the scale of the works being undertaken, they may need to be comply with The Building Regulations 2010 and the [Construction \(Design and Management\) Regulations 2015](#).

Part 1: A brief history of the development of the school estate

Overview

The school estate in England comprises around 64,000 teaching blocks across over 22,000 schools. The size, age of buildings and type of construction varies considerably across the estate.

The following sections show how wider educational policy and social factors affected the types of construction used.

Late 19th century

Education became compulsory for children aged between 5 and 10 years of age in 1880 and up to the age of 12 by 1899. This resulted in a surge in demand for new school buildings. Schools constructed in this period were commonly 3 or 4 storeys high to fit the necessary accommodation on tight, urban sites. Generally, they were constructed in brick and had steep pitched roofs and large timber sash windows.

Early 20th century

Under the 1902 Education Act, responsibility for school buildings fell to local authorities (LAs). A falling birth rate led to a decline in building elementary schools, with new schools mainly

concentrated in the expanding suburbs. Concerns about children's health led to calls for better daylighting and ventilation, and access to fresh air. This started a move away from planning classrooms around central halls to more linear arrangements, with classrooms connected by long, broad corridors or external, covered verandas.

Inter-war years

A lack of school places immediately after the First World War was further exacerbated by a steep increase in the birth rate during the 1920s and a shortage of building materials, labour and funding. As a result, many emergency classrooms were created using buildings originally designed to be wartime huts.

Due to the economic situation in this period, the Baines Committee recommended in 1925 to build school buildings more economically, suggesting that halls and corridors be reduced in size and roofs built to a lower pitch.

Post World War II

There was an urgent need for school building in the period immediately after World War II (WWII). One in five schools in England had been destroyed or badly damaged and there was a rising birth rate. Furthermore, the school leaving age was raised from 14 to 15 years in 1947 as part of the 1944 Education Act which made secondary schooling a statutory requirement.

The unprecedented demand for new school places, alongside post-war shortages of materials and labour, led to the Hutting Operation for the Raising of School-Leaving Age (HORSA huts), which lasted from 1944 to 1950. These were temporary buildings and are covered in more detail in Part 2 of this guidance.

While HORSA huts had met an immediate need, material shortages continued in the years after the war. Expenditure cuts led to all LAs being required to produce carefully costed, 3-year building plans. To achieve cost savings, a series of prefabricated 'industrialised' systems came to the market. These used innovative techniques to minimise material costs and construction times. In the 20 years following the war, up to 80 different 'system-builds' were developed using a variety of steel, timber or precast concrete frames with lightweight cladding panels and partitions.

By the mid-1970s some shortcomings in school building systems were becoming apparent. They had not achieved the economies sought and emerging technical issues were causing concern. Local authorities started looking at other options, using steel frames or traditional materials, and by the mid-1970s load bearing brickwork became almost universal in school building. However, 'system builds' had achieved some significant success in standardising an approach to school building design and provided good quality education space for thousands of pupils.

Towards the Millennium

The 1900s and 2000s saw a significant change in how school buildings were owned, and new schools delivered. For almost a century, schools had been owned and run by local authorities. This changed with the introduction of academy trusts who were independent from local authorities and responsible for the maintenance of school buildings. As well as locally delivered schools, the Department for Education also began delivering construction projects, most recently through the department's own centralised programmes such as free schools, Priority School Building Programme and the School Rebuilding Programme.

The future of construction

In recent years, with continued productivity, supply and resource constraints, government initiatives have promoted the use of Modern Methods of Construction (MMC). MMC solutions range from panelised components to full volumetric 'modular' solutions. Limited by transport constraints, full volumetric MMC solutions are often better suited to cellular spaces such as classrooms.

Part 2: Overview of common construction types in the school estate

Traditional construction

Overview

The term 'traditional construction' is used for buildings with load bearing masonry walls (usually brick), timber (occasionally concrete) floors and timber pitched roofs clad with tiles or slates. Traditional construction has been used for hundreds of years and is still in use today. Traditional construction was the mainstay for school buildings until WWII. The information in this section specifically relates to pre-WWII construction.

Cavity wall construction became increasing widespread from the 1930s. Masonry walls prior to this period were typically solid brickwork. Solid brickwork can usually be identified by the brick patterns used in the walls, with 'headers'¹ evident. It is important that walls are kept in good condition as otherwise water can penetrate causing deterioration of the masonry, supported members and the building fabric more widely.

Timber has always been seen as an inexpensive and easily accessible construction material, although timber shortages and

forestry prices caused by both world wars limited the potential for widescale structural use.

Keeping timber dry is paramount to its durability. It is particularly susceptible to damage where it meets external materials. Damage can also occur at connections between timber and other structural members, or where structural timber has been notched or drilled to accommodate building services.



Figure 1: Example of a traditional pre-war school building constructed from masonry with a pitched roof and chimney features

¹ A brick that is laid flat and perpendicular to the wall's face.

Structural summary

Vertical load bearing elements

In smaller spaces (e.g., classrooms and offices), timber floorboards typically span across timber floor joists which are commonly built into recesses (or 'pockets') within load bearing masonry walls. For larger open spaces (e.g., halls), timber floor joists may be supported by iron or steel beams. In some cases, iron or steel may have been used as isolated columns, although load bearing masonry walls are more common.

Lateral stability

As well as supporting vertical loads (i.e., the weight of structural materials, building occupants etc), masonry walls are required to support loads applied by wind and rely on floors or roofs for restraint to prevent these walls from falling inwards or outwards. It is therefore crucial that any new holes or openings being formed in walls (e.g., for windows) or floors next to walls (e.g., for staircases) are checked by a qualified building professional who should assess the impact on the building structure.

Common foundation types

The type of foundations will depend on the type of soil under the building(s). However, these are likely to be at a shallow depth as brick or concrete 'strips'.

Common material types and maintenance items

The following materials are commonly found in school buildings that use traditional construction:

- masonry (e.g., clay brickwork, stonework or rubble fill)
- timber
- iron/steelwork
- concrete

For traditional construction the following areas should be given special consideration for targeted maintenance:

- stability of chimney stacks or parapets
- condition of brickwork pointing
- location and condition of damp-proofing
- mechanical damage to timber joists
- overloading of timber floors
- water ingress or moisture damage to roofs or floors
- historic unplanned structural modifications

Further details are available in Appendices B and C.

Further information (external links)

- [Historic Structural Steelwork Handbook](#)
- [Historic England Looking After Historic Buildings](#)

Post-war temporary accommodation (HORSA)

Raising the age that children left school and the need for rapid rebuilding in urban areas after the war led to the Hutting Operation for the Raising of the School-Leaving Age (HORSA) policy. HORSA buildings were distinctive one storey prefabricated constructions that were used for education. These were intended to be temporary buildings constructed in concrete and timber, with metal windows and corrugated asbestos roofs. They are sometimes referred to as Pratten Huts as they were commonly supplied by F. Pratten and Co Ltd.

At the time of construction, concrete often inadequately covered steel reinforcement, leaving a potential for the reinforcement to corrode over time and result in the 'spalling'² of the concrete. Similarly, poor detailing left timber elements prone to deterioration due to frequent exposure to moisture (e.g., condensation) and/or the elements.

HORSA huts are beyond their expected lifespan and require careful maintenance.

² Spalling refers to the process of surface failure due to large shear stresses under the surface caused (in this case) by corroding steel reinforcement.



Figure 2: Example of a simple, single-storey prefabricated HORSA hut with a lightweight structure and shallow roof

Structural summary

Vertical load bearing elements

Roof sheets, sometimes constructed of asbestos, are supported by roof beams, commonly formed of timber. The roof structure is supported on the perimeter walls, which are typically concrete and in some cases timber stud work.

Lateral stability

As well as supporting vertical loads (i.e., the weight of the roof and potential snow loads etc), the concrete panel walls are required to support loads applied by wind.

Common foundation types

As these temporary buildings were single storey, they would commonly have been founded on very shallow foundations, most likely concrete trenches and potentially, no foundations at all.

Common material types and maintenance items

The following materials are commonly found in HORSA buildings:

- concrete
- timber
- asbestos

For post-war HORSA buildings the following areas should be given special consideration for targeted maintenance:

- management and risk assessment of asbestos – refer to link under ‘Further information (external links)’ below
- deterioration of timber elements due to water damage (either from water penetration or condensation)
- deterioration of concrete elements due to water damage (either from water penetration or condensation)

Further details are available in Appendix B.

Further information (external links)

- [Designing Buildings: HORSA hut](#)
- [GOV.UK Guidance on managing asbestos in your school or college](#)

Post-war 'system builds'

Overview

Numerous forms of prefabricated school building systems were developed after WWII. The demand for replacing war-damaged buildings and for school places due to the soaring birth rate led to a boom in 'system build' construction.

The term 'system builds' is commonly used for these post-war prefabricated buildings. Manufacturers developed bespoke systems comprising a mixture of concrete, steel and timber elements. There was a significant drive to off-site, industrialised, manufacturing and improved on-site construction speeds.

'System build' manufacturers were either:

- private firms: developing their systems for housing and then adapting them for other building types
- 'consortia' of local authorities: developing systems which allowed efficient delivery due to economies of scale

In some cases, the consortia developed a single system, for example CLASP (with its distinctive bracing design). In other cases, they developed a range of solutions using different materials, for example SEAC (steel, concrete and masonry systems). Private suppliers included Orlit and Derwent (also known as Vic Hallam). Further details of these systems are provided in Appendix A.



Figure 3: Example of a post-war CLASP system school building with a modular frame

Structural summary

Vertical load bearing elements

The vertical load bearing elements varied depending on the system being used. Appendix A provides further information on some of the common types of system, allowing the reader to identify whether they have a system build and potential areas where maintenance should be focused.

Lateral stability

The elements required to provide horizontal stability varied depending on the system being used. Again, Appendix A provides further information on some of the common types of system, allowing the reader to identify whether they have a system build and potential areas where maintenance should be focused.

Common foundation types

The foundation type(s) will depend on the ground conditions present on the site. Some systems had bespoke foundations to suit the expected ground conditions in the areas where the consortia were based. For example, foundations to CLASP buildings were designed to accommodate movement due to mine workings.

See Appendix A for system specific foundation details.

Common material types and maintenance items

The following materials are commonly found in school buildings that use post-war 'system builds':

- masonry
- timber
- lightweight steelwork
- concrete

For post-war 'system builds' the following areas should be given special consideration for targeted maintenance:

- cracking or movement at joints between precast concrete panels
- timber elements for signs of deflection (bowing)
- rust or corrosion of steelwork elements

Further information about specific systems can be found in Appendix A and common construction components in Appendix C. Information concerning common materials can be found in Appendix B.

Further information (external links)

- [Historic England: Historic School Buildings](#)
- [Historic England: England's Schools 1962-88](#)

Post-war non-systemised construction

Overview

In the late 1960s and early 1970s, many consortia began to disband. By the mid-1970s there was mostly a return to traditional construction methods which commonly used masonry walls. However, CLASP and other systems were still being developed and produced.

New and innovative materials were also in use, primarily for floors as alternatives to traditional timber construction. Products such as Orlit, Bison and other precast concrete forms, as well as woodwool, were used for floors as well as flat and occasionally pitched roofs. Further details and common forms of these products are provided in Appendix C.

Masonry buildings typically comprised cavity wall construction, whereby two 'leaves' of brickwork are built with a small, predominantly 50mm at the time, gap between them to improve the insulation properties of the wall when compared to earlier solid brick walls. Masonry leaves were tied together using steel ties which latterly became stainless steel to avoid problems with rusting/corrosion.

These non-systemised buildings were designed by several engineers and architects meaning their shape and appearance vary. However, the majority consist of rectangular blocks with flat roofs.

Structural summary

Vertical load bearing elements

The floor and roof construction of these buildings vary. In some cases, timber plywood sheets spanned across timber joists onto steel beams. In other cases, precast concrete panels were supported by steelwork. In many cases, load bearing masonry cavity walls support the floors and roofs directly.

Lateral stability

The masonry walls to the perimeter of the building and internal 'cross walls' resist the loads applied by wind. It is therefore crucial that any new holes or openings being formed in walls are checked by a qualified building professional.

Common foundation types

Foundations will be designed to suit the ground conditions present on the site. Given that load bearing masonry is not a lightweight construction, buildings more than one storey, or on soft ground may have deeper, piled foundations.

Common material types and maintenance items

The following materials are commonly found in school buildings that use post-war non-systemised construction:

- masonry
- timber
- concrete
- woodwool or stramit

For post-war non-systemised buildings, the following areas should be given special consideration for targeted maintenance:

- bulges or distortion of masonry walls which may indicate missing or corroded walls ties
- signs of water ingress, particularly around flat roofs or perimeter walls/parapets

Further details are available in Appendices B and C.

Further information (external links)

- [NHBC House Building Standards 2024](#)³

³ Note – these are the current standards for masonry in housebuilding. While the details may not be directly applicable to post-war non-systemised

Reinforced Autoclaved Aerated Concrete

Reinforced Autoclaved Aerated Concrete (RAAC) was produced in the UK between the 1950s and early 1980s and was used in some post-war non-systemised construction. It is a lightweight, 'bubbly' form of concrete predominantly found as precast panels in roofs (mostly flat, sometimes pitched) and occasionally in floors and walls.

The DfE has completed its identification programme, which asked all Responsible Bodies (local authorities, academy trusts, dioceses, and further education colleges) to look for RAAC in their buildings.

If you believe that a premises may contain RAAC or if you are unsure, you should seek assistance from an appropriately qualified building surveyor or structural engineer with experience of RAAC identification. If RAAC is confirmed to be present, and you are a responsible body for a state-funded education setting in England, you should inform the DfE using the [Customer Help Portal](#). An official will make contact once you have completed the form to discuss next steps.

construction, they give an indication of the details and construction that could be identified during inspection.

Framed construction

Overview

Framed construction became increasingly popular in the school estate in the late 20th and early 21st century. This form of construction allows greater flexibility of internal spaces and less repetition in the external appearance of buildings.

In framed construction, the external cladding is generally non-load bearing so could comprise materials other than brick.

The use of discrete load bearing columns and lightweight, non-load bearing partitions, means internal layouts can be altered to suit future changes in educational practices without the need for significant structural alteration. The use of framed construction also gives the opportunity to introduce larger atria and double height spaces in circulation areas.



Figure 4: Example of a steel-framed school building



Figure 5: Example of a concrete-framed school building

Structural summary

Vertical load bearing elements

Concrete framed buildings typically comprise floors and roofs formed of concrete, either as precast panels or in-situ slabs.

Floors of steel framed buildings typically have precast concrete slabs or in-situ concrete poured onto a profiled metal deck. In some cases, roofs are lightweight profiled metal deck, and in older buildings may have been formed using woodwool panels. Further details are available in Appendix B. In both steel and concrete framed structures, the floors and roofs are supported by discrete columns.

Lateral stability

Wind loads are commonly resisted by 'bracing' elements. In concrete frames, these are likely to be concrete 'shear' walls. In steel framed buildings, these may be diagonal bracing. Should further information be needed about the type of lateral stability of a building you should contact a suitably qualified and experienced chartered engineer who will be able to advise.

Common foundation types

The foundation types will depend on the size and layout of the building as well as the ground conditions present on the site. Should you require further information about a building's foundation you should contact a qualified building professional who will advise whether any on-site investigations are needed.

Common material types and maintenance items

The following materials are commonly found in school buildings that use framed construction:

- steel
- concrete
- masonry
- timber

For framed construction the following areas should be given special consideration for targeted maintenance:

- signs of water ingress e.g., through flat roofs
- gutters and downpipes being clear, so rainwater is directed away from the building
- flashings, roof coverings etc are not damaged, especially after adverse weather
- signs of cracking or movement in façade panels

Further details are available in Appendix B.

Further information (external links)

- [Cross UK: Collaborative Reporting for Safer Structures UK](#)
- [Concrete Building elements](#)
- [Designing Buildings](#)

Modern Methods of Construction (MMC) / Design for Manufacture and Assembly (DfMA)

Overview

MMC/DfMA is a collective term used to describe construction practices that are alternative to traditional building. It focusses on the use of off-site construction techniques within factory settings to produce building components for transportation to, and assembly on, site. It includes:

- preformed panels (typically steel or timber) which are connected on site to form walls, floors and roofs
- preformed 'volumetric' units i.e., 'boxes' with some or all components already installed which are bolted together on site
- hybrid techniques that combine the above

MMC/DfMA cover a wide range of construction materials and practices which are beyond the scope of this document. Please refer to the links in 'Further information (external links)' below for more details.

Further information (external links)

- [MMC Definition Framework](#)
- [GOV.UK: The Construction Playbook](#)
- [Go Construct: Modern Methods of Construction \(MMC\)](#)



Figure 6: Example of a completed modular school building using modern methods of construction



Figure 7: Example of off-site manufacture of modular building units within a factory environment

Appendix A: Further information on common post-war ‘system builds’

Introduction

This appendix provides information about the typical features that can be used to identify the most common types of post-war ‘system build’ in the school estate.

Based on the information in this appendix, the reader should be able to identify if they have buildings of these types and then use the visual guides to identify areas that should be given special attention and consideration for targeted inspection and maintenance.

Derwent



Figure 8: Example of a Derwent system school building with panelised external cladding

Background

In the 1950s, Morrison Design Architects were invited by Derbyshire County architect, Hamer Crossley, to design a new school utilising an Austrian system concept of timber prefabrication. They proposed a UK based timber panelised construction which could be used across the country.

Uses: Primarily schools. Variations include offices, clinics, hostels, libraries, gymnasia, community, health & welfare centres

Structural frame type: Timber

Approximate years of manufacture: 1952 - 1963

Manufacturer: Vic Hallam Ltd, Nottingham

Architect: Morrison Design

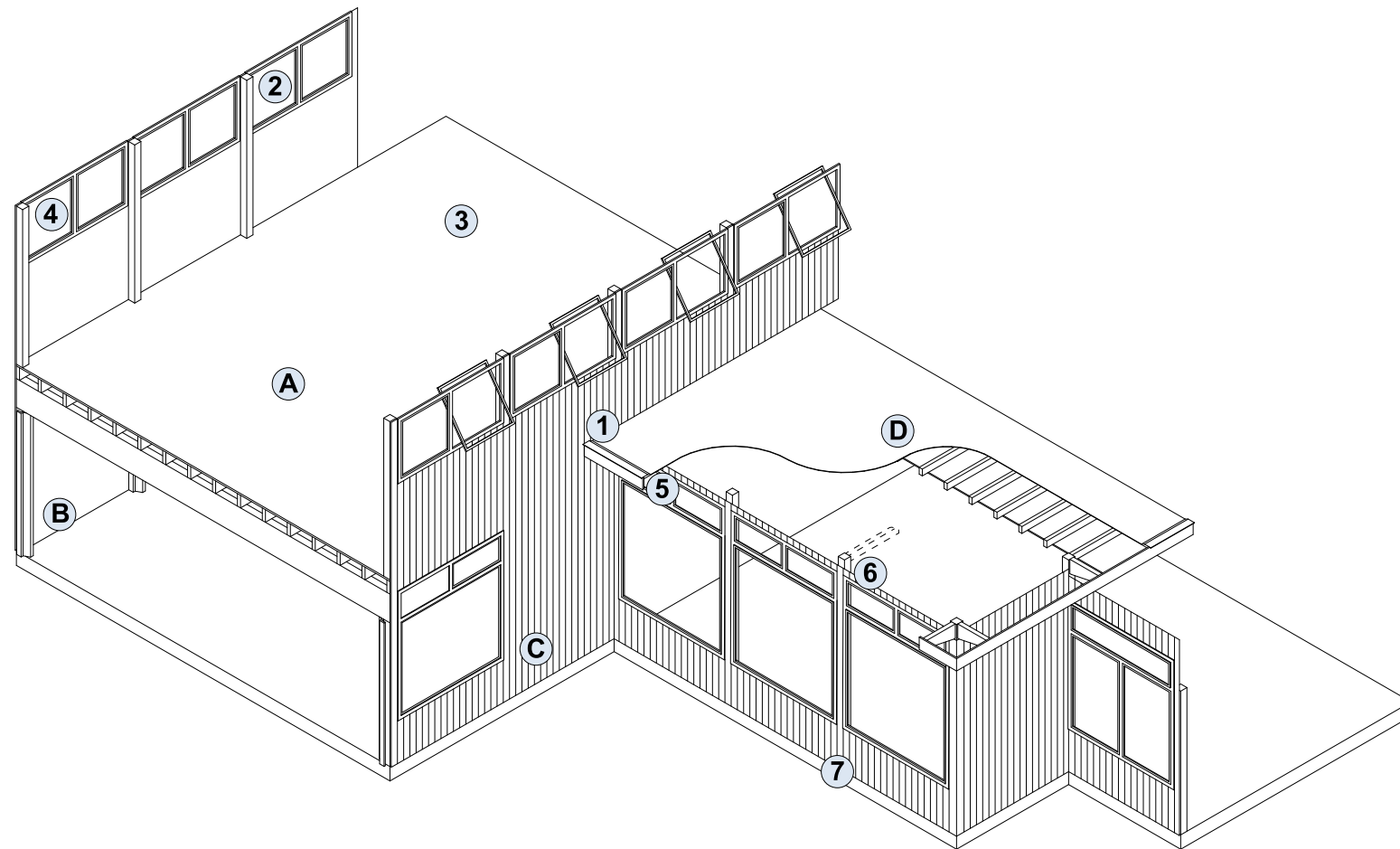
Known locations (though likely to exist elsewhere): Over 30 regions including Sheffield, Birmingham, Yorkshire, Essex, Derbyshire, Lancashire, Nottinghamshire and Warwickshire

Identifying Derwent construction

Foundations	Vary to suit ground type/conditions
Ground floor	Concrete
Upper floors	Timber plywood on battens
Roof	Timber plywood on battens with 12° pitch
Vertical structure	Timber columns with some load bearing timber panels
Horizontal stability	Infill timber panels
Planning grid	6' 4" (1930mm)
Structural grid	6' 4" (1930mm)
Planning height	1' 4" (406mm) increments
Typical column size	4" (102mm)
Typical beam size	Varies

Features and common maintenance items

The sketch below outlines the defining characteristics of a Derwent building and indicates where inspection should be carried out to identify common maintenance items with this construction type. The items shown should be cross referenced against Part 2 and Appendix B of this document.



Identifying features:

- 1 Cladding materials: vertical timber boarding, brick, stonework and sheet materials (e.g., plywood)
- 2 Panel module: 6' (1.8m) panel width and 4" (100mm) column
- 3 Plywood sheeting
- 4 Column details - see overleaf
- 5 500mm overhanging eaves
- 6 Column roof purlin connection
- 7 Indicative location of column base

Common considerations:

- A Decay of columns leads to movement of beams and uneven floors
- B Wet rot at base of column due to water ingress
- C Decay of timber columns can lead to column failure, distortion of external elevations and bulges in the exterior timber cladding
- D Wet rot to roof structure due to water ingress

Figure 9: Axonometric of a Derwent system school building showing its timber framing and panelised external cladding

Typical details and common maintenance items

Column profile:

Columns on the ground floor of a single storey building and first floor of a two-storey building are formed from small timber sections, typically four no. 4" x 1" timbers and are susceptible to decay.

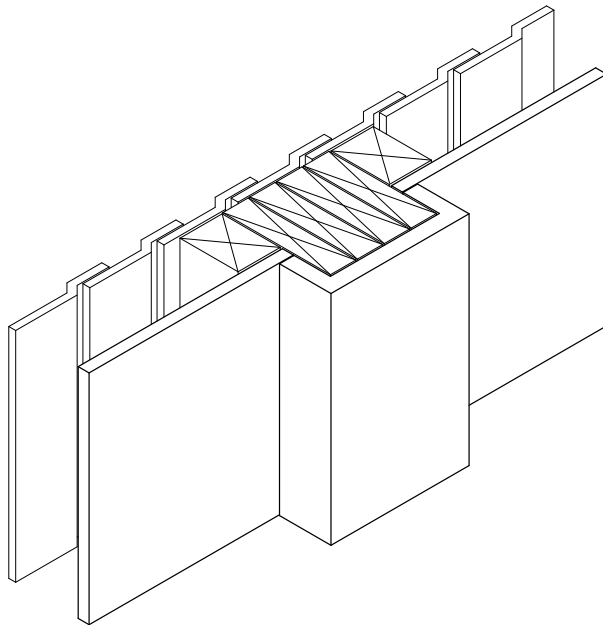


Figure 10: Detailed view of a timber column profile used in the Derwent system

Column profile with tapered fin:

Columns on the ground floor of a two-storey building are likewise formed from small timber sections and have an additional tapered fin attached to the main column to support the first-floor beam.

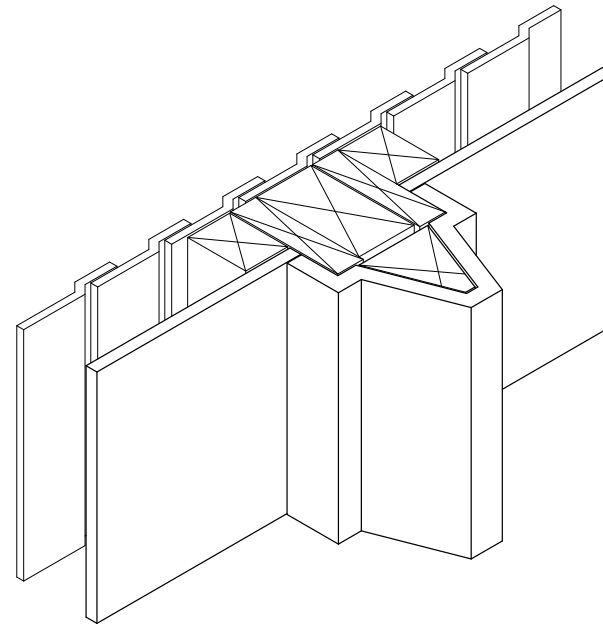


Figure 11: Detailed view of a timber column profile with an added tapered fin

Service run identifying feature:

Service wire route passes from behind the skirting, up through a continuous hollow service run within the timber column and roof purlin construction.

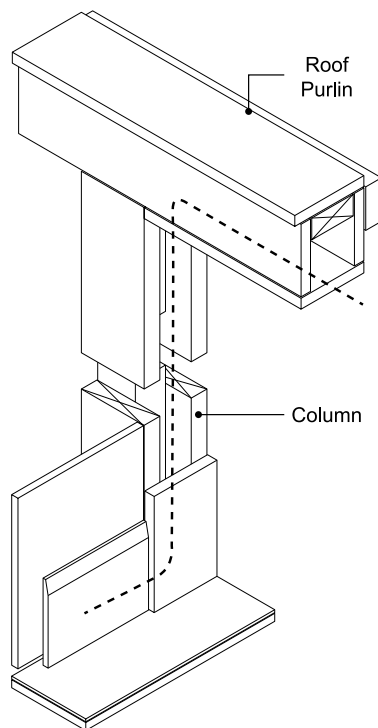


Figure 12: Diagram of an internal service route through a structural column

Eaves detail identifying feature:

Typical 500mm overhanging eaves with timber fascia, external rainwater pipes and $\frac{3}{16}$ " plywood soffit.

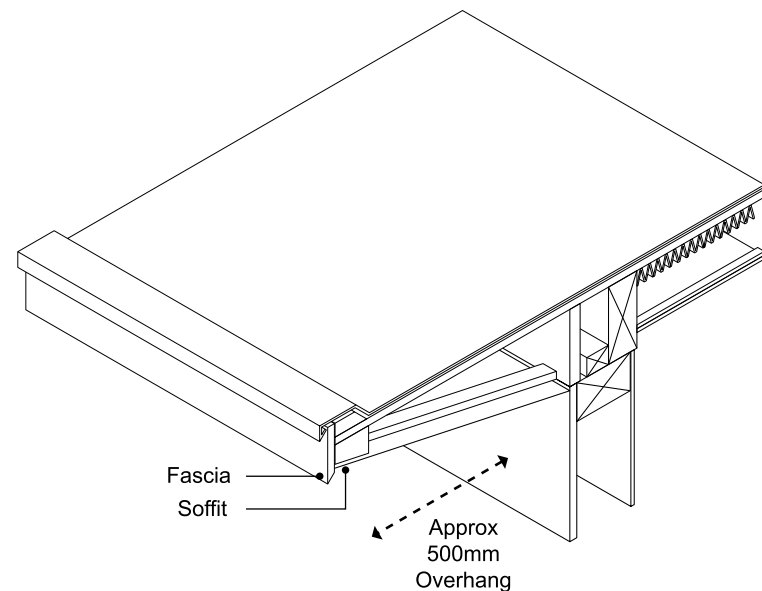


Figure 13: Detailed view of a roof eaves including overhang and fascia

Floor beam: typical identifying feature:

Solid laminated timber floor beam with cross battens supporting plywood floor. Cellular plasterboard soffit secured to underside of battens and beams encased with fibrous plaster.

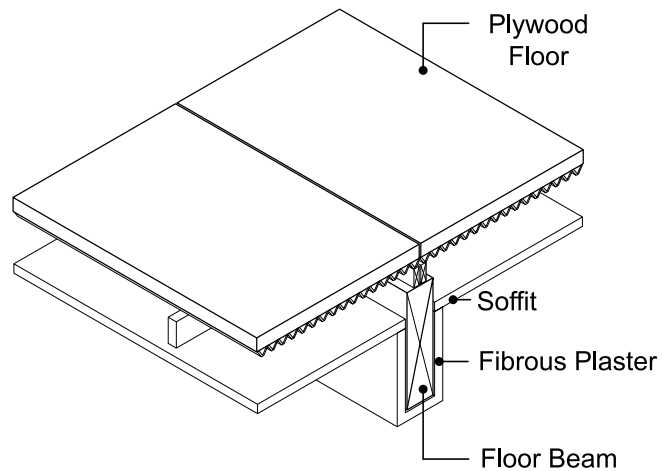


Figure 14: Detailed view of a timber floor including supporting beam

References and further information

- [Morrison Design Architects Website](#)
- System building: the 'Derwent' system of Vic Hallam Ltd: RIBA Journal 1969 July, p287-291
- Sheffield sparks Derwent system alert: Architects Journal 1987 Oct 21, p13
- The Derwent System of Construction: Vic Hallam Oct 1972
- The Derwent system of building: Prefabrication, Vol.2, May 1955, p311-317
- Derwent construction for schools: The Architect and Building news, 5 Aug 1954, p151-161

Orlit



Figure 15: Example of an Orlit system school building with a precast concrete frame and panel construction



Figure 16: Example of an Orlit soffit

Note: Slabs are not always imprinted with 'Orlit'.

Background

Orlit was the design concept of Czech architect, Erwin Katona. It was originally designed as a precast concrete framed structure for housing and externally clad with 2-inch reinforced concrete hollow blocks. Orlit houses are classified as 'defective'⁴ in England and Wales and have typically been subject to extensive repairs or demolished.

Uses: Primarily houses and flats. Known variations include schools, libraries, health centres, civic buildings, factories, church halls and offices

Structural frame type: Precast reinforced concrete

Approximate years of manufacture: 1940s & 1950s

Manufacturer: Orlit Ltd, Vicarage Road, Egham, Surrey

Architect: Erwin Katona

Known locations (though likely to exist elsewhere):

Birmingham, Brighton & Hove, Bromley, Coventry, Croydon, Essex, Hackney, Havering, Kent, Lancashire, Lincolnshire, London, Newcastle, Redbridge, Somerset, Stoke on Trent and Woolwich

Identifying Orlit construction

Foundations	Concrete
Ground floor	Concrete
Upper floors	Timber plywood on battens, on secondary concrete beams
Roof	Precast concrete slabs on precast concrete secondary beams, with screed to falls and asphalt or bituminous felt covering. Pitched roof option: timber or steel structure, tiled finish
Vertical structure	Precast concrete columns
Horizontal stability	Precast concrete secondary beams or (where there is a concrete floor) precast inverted channel floor slabs
Planning grid	Variable
Structural grid	Columns at 10' (3050mm) or 12' (3660mm) centres
Planning height	Variable
Typical column size	5½" square (140mm)
Typical beam size	Primary: 8" depth (200mm) Secondary: 7" x 4" (178 x 102mm) at 4' (1220mm) centres

⁴ The Housing Defects Legislation (now Part XVI of the Housing Act 1985).

Typical details and common maintenance items

Structure

Precast reinforced concrete columns at either 10' (3.050m) or 12' (3.660m) centres and beams.

Maintenance items include:

- structural weakness due to cracked main and secondary beams and columns, caused by reinforcement corrosion (this is generally associated with flat roof forms)
- failure of column/beam connections, generally due to condensation
- variable bearing lengths at junction between precast concrete beams due to construction tolerances

Cladding

Precast concrete facing slabs 4' wide x 1' 4" high (1220mm wide x 405mm high) and 2' wide x 1' 4" high (610mm wide x 405mm high). Factory finished with various textures and colours. Other types of cladding may also have been used.

Maintenance items include:

- cracking and spalling of concrete cladding, and other non-structural components

Internal cladding/leaf

Lightweight 2½" (64mm) thick concrete blocks.

Floors

Timber plywood on battens, on secondary concrete beams. Or, where there is a concrete floor, precast concrete inverted channel floor slabs.

Roof

Precast concrete slabs 4' x 1' x 2" thick (1220mm x 305mm x 50mm) on precast concrete secondary beams, with screed to falls and asphalt or bituminous felt covering.

Pitched roof option: timber or steel structure with a tiled finish.

Common maintenance items:

- flat roof construction susceptible to condensation leading to deterioration of the high alumina cement joints

References and further information

- The structural condition of Orlit houses: BRE Report 1983
- [Orlit House](#)
- Deterioration and repair of prefabs: RIBAJ 1984 vol 91, no.2 Feb, p60
- Non-traditional housing in N. England, NCHA

CLASP



Figure 17: Example of a CLASP system school building with steel frame and external cladding panels

Background

CLASP is the first example of a group of local authorities forming a voluntary consortium to develop and control a system of construction for their own use. Founder members were the City of Coventry, Derbyshire County Council (CC), Durham CC, Glamorgan CC, West Riding of Yorkshire CC, the City of Leicester and Nottinghamshire CC. CLASP was considered particularly useful in coal mining areas at risk from mining subsidence where it provided schools at little extra cost. As the design evolved over the years, new 'Marks' (Mks) were released (see CLASP Marks, pages 36 - 38).

Uses: Houses and flats, schools, libraries, health centres, civic buildings, factories, church halls and offices

Structural frame type: Pin jointed steel

Approximate years of manufacture: 1957 - 2005

Manufacturer: Blockhouse Steel Structures Ltd

Architect: Donald Gibson (Nottinghamshire CC), further developed later with Charles Aslin (Hertfordshire CC)

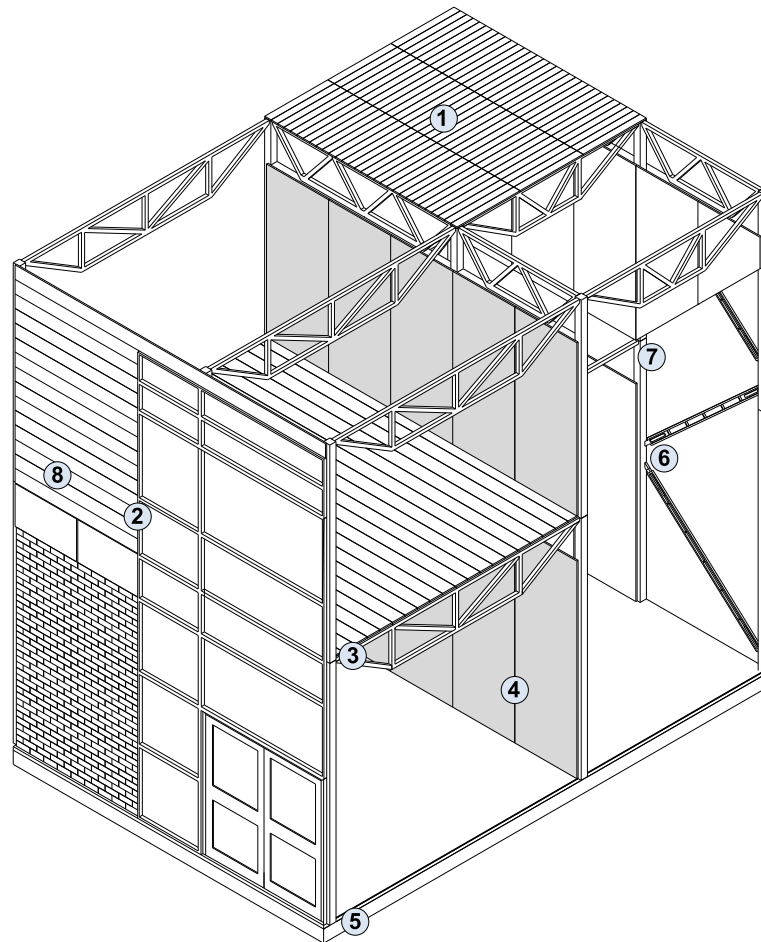
Known locations: There were over 1,400 CLASP schools in the UK, distributed among 89 Children's Services Departments. Independent schools own a small number of CLASP buildings

Identifying CLASP construction

Foundations	Concrete
Ground floor	Screed on concrete slab and floor finish (woodblock or plastic tiling)
Upper floors	1½" x 4" (38 x 102mm) t&g timber boarding spanning 3' 4" (1016mm) between steel secondary beams
Roof	Prefabricated timber panels - ¾" thick (19mm) on 5" x 1½" (127 x 38mm) and 4" x 2" (102 x 51mm) joists, and 3 layers bituminous felt with ½" (13mm) gravel
Vertical structure	Cold rolled steel strip welded to form box sections
Horizontal stability	Steel primary beams, secondary beams (welded lattices of hot-rolled and cold-rolled angle sections with flat lower chord) and diagonal ties (fixed and cold rolled steel or spring-loaded units)
Planning grid	Horizontal: 3' 4" (1016mm), vertical: 4" (102mm)
Structural grid	Column centres at 3' 4" (1016mm) or multiples thereof
Planning height	Stanchions: 10' (3048mm) to 24' (7315mm) in 2' (610mm) increments
Typical column size	4½" x 4½" (114mm) square steel sections
Typical beam size and span	Spans from 3' 4" (1016mm) to 26' 8" (8128mm)

Features and common maintenance items

The sketch below outlines the defining characteristics of a CLASP building and indicates where inspection should be carried out to identify common maintenance items of this construction type. The items shown should be cross referenced against Part 2 and Appendix B of this document.



Identifying features:

- 1 Roof: prefabricated timber panels formed of $\frac{3}{4}$ " boards nailed to joists with 3 layers bituminous felt & $\frac{1}{2}$ " gravel chippings
- 2 Window cills set at 2', 2' 8" or 3' 4" above finish floor level (FFL). Transoms and door heads at 6' 8" above FFL
- 3 Floor finish: $1\frac{1}{2}$ " x 4" t&g boarding spanning 3' 4" between steel secondary beams
Consideration: Rotten timber floors and roofs, particularly in earlier Marks
- 4 Partition walls of prefabricated plasterboard units consisting of 2 sheets plasterboard and a paper core (overall thickness $2\frac{1}{4}$ "). Other options include: Parana pine boarding, hardboard, asbestos board
- 5 Column bases cast at intervals of 3' 4" (or multiples of)
- 6 Spring loaded windbracing
- 7 Column beam connection
- 8 Outer cladding typically formed of either pre-cast concrete units (thickness varying from $2\frac{1}{2}$ " at ribs and edges to $1\frac{1}{4}$ " in the centre), clay tiles, horizontal timber boarding or corrugated aluminium sheets

Figure 18: Axonometric of a CLASP system school building showing its structural frame and components

Typical details and common maintenance items

Common maintenance items:

Asbestos containing materials (ACMs) are found in many locations including boarding around steel members, shuttering, packers and panels, service ducts, pipe lagging, cisterns, duct covers, blind boxes to the window frames and floor coverings. Asbestos fibres can escape from:

- poor seals at joints or gaps in column casings when subject to impact e.g., by furniture or closing an adjacent door
- disturbance caused by minor works e.g., cabling, replacing windows etc
- unsealed column tops in ceiling voids
- asbestos debris left in columns, external wall cavities and ceiling voids

Alterations to the slab and structural frame will have a significant impact on structural integrity.

Acoustic and thermal performance is difficult to upgrade, underfloor insulation is generally not possible.

Timber floors and roofs, particularly of earlier Marks, are susceptible to rot and may require total replacement.

Windows tend to be ill-fitting.

Column/cladding detail identifying feature:

Outer cladding secured via battens to cladding frame. Cladding typically formed of either precast concrete units (1' 4" (400mm) high and available in various textures and colours), clay tiles, horizontal timber boarding or corrugated aluminium sheets. Cladding frame formed from 5⁵/₈" x 1¹/₂" (143 x 38mm) softwood sections, available in 6' (1830mm) and 9' 4" (2845mm) widths.

Columns set within cladding; considerations include cracking in external cladding and uneven floors.

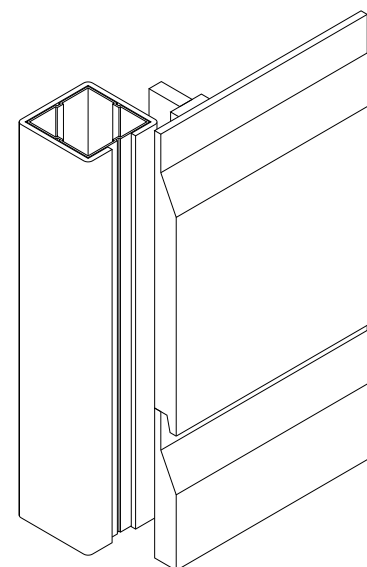


Figure 19: Detailed view of a CLASP column and its connection to external cladding

Column/beam connection identifying feature:

The structural framework is composed of pin-jointed steel columns, beams and diagonal ties. Steel columns are formed from $4\frac{1}{2}" \times 4\frac{1}{2}"$ (114 x 114mm) cold rolled steel angle sections welded to form box sections. Steel beam connection plates are welded to columns.

Considerations include failure of column/beam connections.

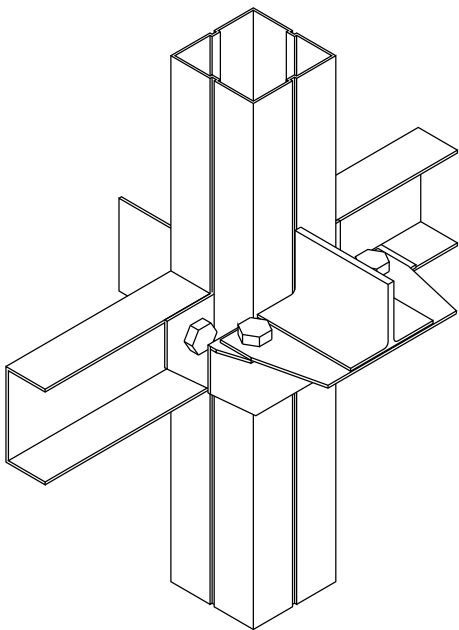


Figure 20: Detailed view of a column and beam connection used in a CLASP structure

Wind bracing identifying feature:

Spring loaded steel bracing incorporating a pair of heavy coil compression springs was unique to CLASP. This is generally concealed within external walls and partitions but is occasionally exposed.

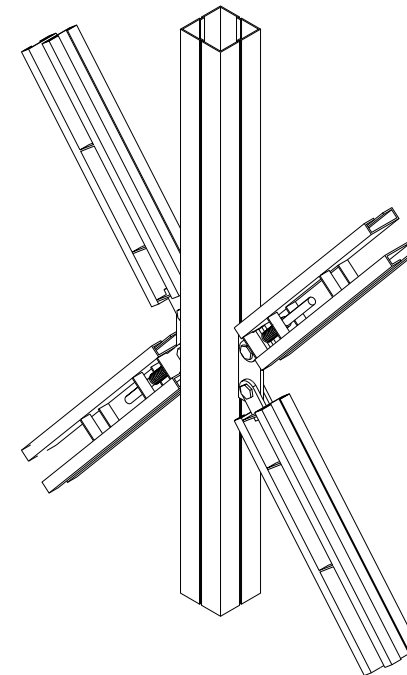


Figure 21: Detailed view of the wind bracing system used in a CLASP structure

CLASP Marks: Identifying characteristics

CLASP Mk 1: 1958/60

Mark 1 was not an official name. The system was at an early stage of development with each authority producing their own drawings. Characteristics include:

- Maximum storeys: 3.

CLASP Mk 2: 1958/60

Mark 2 was the first official designation. Characteristics include:

- Cladding: Horizontal concrete panels: 1' 4" (406mm) or 8" (203mm) high and 6' 8" (2032mm) or 10' (3048mm) wide; and tile hanging.
- Roof eaves: Projecting eaves with timber fascia; external rainwater pipes; aluminium pressing incorporating drip secured to top of fascia.
- Windows: Good quality painted timber windows. Top hung vents with some louvres also available.
- Ceilings: Fibrous plaster tiles and plasterboard.

CLASP Mk 3: 1960/62

Characteristics include:

- Cladding: Early Mark 3 concrete cladding units were as Mark 2. Late Mark 3 concrete cladding units were either 3'

4" (1016mm) or 2' 8" (812mm) wide by 2' (610mm) high. Tiles were commonly used for cladding with a wide range of tiles available.

- Roof eaves: Timber fascia is now set back in line with face of building with aluminium angle secured to top of fascia and external rainwater pipes.
- Windows: Horizontal sliding windows in mill finish aluminium.
- Ceilings: Fibrous plaster tiles and plasterboard as Mark 2, with Asbestolux as an option.
- Partitions: Asbestolux an option, mainly as internal lining to external walls.

CLASP Mk 3B: 1962/65

Mark 3B is similar to Mark 3. Main differences are in steelwork design, including the introduction of a cambered, or slightly arched, roof truss to provide a fall for rainwater, and a better range of windows. Characteristics include:

- Cladding: 2' (610mm) high concrete units introduced, width 3' 4" (1016mm) or 2' 8" (812mm). Tile hanging, timber boarding and profiled aluminium sheet also available.
- Windows: As Mark 3, except louvres replaced by top hung vents.
- Roof: Cambered roof truss introduced to provide fall. At the eaves, an extruded aluminium section was used as

Appendix A

the edge trim to top of fascia, replacing the simple aluminium angle.

CLASP Mk 4: 1965/69

Typically used for residential university buildings where smaller rooms needed. Characteristics include:

- Module: 4" (102mm) module used but in multiples of 9 (i.e., 3' (915mm), 6' (1830mm) etc). An additional 1' (305mm) unit was available.
- Maximum storeys: 4.
- Structural floor zone: 2' (610mm).
- Ceilings: Mineral fibre tiles, plasterboard or Asbestolux. Tiles: Typically 2' x 4' (610 x 1220mm).
- Cladding: Vertical format concrete cladding unit, 8' (2438mm) or 10' (3048mm) high (storey heights) by 2' (610mm) wide with 1' (305mm) and 3' (915mm) options. Other sized units for under cills, eaves and floor fillers. Tile hanging and timber boarding continue to be used.
- Partitions: Asbestolux used as an internal lining to window units.
- Door frames: Stove enamelled steel.

CLASP JDP: 1968

Designed for higher education as well as primary and secondary schools. Characteristics include:

- Structure: Hot rolled steel and precast concrete.

- Maximum storeys: 6.
- Suspended floor structure: Precast concrete deck spanning 6' (1829mm) on a fabricated lattice of steel main and secondary beams.
- Columns: 6" (152mm) and 8" (203mm) square, cross section reduced to 4½" (114mm) square at connection with beams. Column free bays under floors of 12' x 30' (3,658 x 9144mm) or 18' x 24' (5,486 x 7315mm).
- Horizontal stability system: Diagonal steel wind braces (typically concealed within wall structure). Concrete floor deck and steel structure interlock so that wind forces transmitted through floor to the wind braces.
- Structural floor zone: 3' (915mm) minimum.
- Ceilings: Heights up to 10' (3048mm).
- Staircase widths: Increased to maximum 3.6m.
- Cladding: As Mark 4 cladding.

CLASP Mk 4B: 1969/72

Mark 4B was similar to Mark 4 but designed for higher education as well as primary and secondary schools. Characteristics include:

- Suspended floor structure: Timber with foam backed rubber covering over hardboard.
- Columns: 4" (102mm) square.
- Ceilings: Preferred floor to ceiling heights are 8' (2438mm) and 10' (3048mm) with increased heights in sports facilities etc. 3' x 3' (915 x 915mm) mineral fibre tiles now

Appendix A

in an exposed tee system. Similar size Asbestolux panels also used. 1' x 1' (305 x 305mm) mineral fibre tiles in a concealed system available.

- Cladding: As Mark 4, GRP floor/roof filler panel also available.
- Partitions: Stevetite partitioning used extensively (plastic-faced, steel sheet with plasterboard backing).

CLASP Mk 5: 1972/84

Mark 5 brought the change to metric. Characteristics include:

- Module: Now 100mm with a structural grid of 900mm x 900mm and a planning grid of 300mm x 300mm.
- Cladding: Generally concrete with white, red, grey, brown or green aggregate finish. Also brick cladding trialled in 1972, slate trialled in 1973.
- Roofs: Pitched and hipped roofs from 1976/77.

CLASP Mk6: 1980

The structural grid was increased to 1800mm x 1800mm.

CLASP Mk 6B: 2005

Exposed steel, aluminium finishes and curved roofs.

References and further information

- Building Bulletin 19: The Story of CLASP, Ministry of Education, June 1961
- Building Bulletin 45: CLASP/JDP: The development of a building system for higher education. Department of Education and Science, 1970
- [Designing Buildings: CLASP](#)
- Industrialised Building: CLASP. Diamant R M E
- Identification Guide. The CLASP Development Group

SCOLA

Background

SCOLA (Second Consortium of Local Authorities) was an early example of a group of local authorities forming a voluntary consortium to develop a standardised system of construction for their own use. Established in the early 1960s, following initiatives such as CLASP, it was developed primarily to respond to the urgent need for new school buildings following World War II.

The consortium brought together several southern English authorities, working collaboratively to produce buildings that could be erected quickly and economically. Over a relatively short period, large numbers of SCOLA schools were constructed, with examples widely distributed across England.

The SCOLA system used a 'system build' approach based on a lightweight steel frame with standardised components, allowing flexibility in layout while maintaining cost efficiency.

Uses: Primarily schools, with some application in further education and related public buildings

Structural frame type: Lightweight steel frame with prefabricated panel elements

Approximate years of manufacture: 1961 - 1990

Manufacturer: Not tied to a single manufacturer; components procured through consortium-led specifications and competitive tendering

Architect: Developed by participating local authority architects' departments; no single named architect consistently attributed

Known locations: Widely distributed across England, particularly in the south

Appendix A

Identifying SCOLA construction

Foundations	Concrete pad and strip footings
Ground floor	Screed on concrete slab and floor finish
Upper floors	Precast concrete module panels, typically 4' (1220mm) x 2' (610mm)
Roof	Pre-felted woodwool slab decking, joints taped with 3" wide bitumen tape
Vertical structure	Steel frame columns
Horizontal stability	Steel cross-bracing - flat plates vertically between columns and steel bar horizontally between beams below floor structure
Planning grid	Typically 2' (610mm)
Structural grid	Varies in Y-direction, but typically 4' (1220mm) in X-direction to suit floor panel modules
Planning height	Varies, but maximum height circa 22' (6705mm)
Typical column size	6" x 6" (152mm) square steel sections
Typical beam size and span	Varies, see section on steel beams overleaf

Typical details and common maintenance items

Common maintenance items:

SCOLA buildings utilise a lightweight, prefabricated steel frame system with a mix of light and heavy cladding systems. Due to their age, construction methods and material choices, several recurring maintenance issues are commonly encountered, including:

- buildings are typically of modular, bolted construction with numerous interfaces, which can be susceptible to movement and water ingress over time
- many original materials have exceeded their intended design life and may now be experiencing deterioration if not adequately maintained

Maintenance strategies should prioritise weather tightness, structural integrity, and preservation of original components where feasible.

Light cladding (timber/tiles/panels):

Common issues include:

- timber decay, warping, and splitting due to moisture exposure
- corrosion or failure of fixings leading to loose elements
- deterioration of flashings and weathering details

Maintenance items include:

- replacement of decayed timber and damaged cladding units
- refixing loose elements and renewing flashings
- application of protective coatings

Heavy cladding (precast concrete):

Common issues include:

- cracking, spalling, and surface deterioration
- corrosion of embedded fixings
- failure of joints and sealants allowing water ingress

Maintenance items include:

- inspection and repair of cracks and spalled areas
- renewal of sealants and joints
- localised panel repair or replacement where required

Masonry cladding:

Common issues include:

- mortar deterioration and cracking
- movement at interfaces with steel frame

Appendix A

Maintenance items include:

- repointing and localised repairs
- crack stitching as required

Steel frame:

Common issues include:

- corrosion of steel members due to moisture ingress (particularly around external perimeter if defects are present in the cladding or masonry)
- deterioration of protective coatings
- deterioration of bolted connections

Maintenance items include:

- inspection of frame and connections
- cleaning and recoating of steelwork
- tightening or replacement of bolts

Columns and connections:

Common issues include:

- corrosion at base plates and holding down bolts
- degradation of grout and moisture ingress at joints

Maintenance items include:

- inspection and repair of base connections

- regrouting and sealing where necessary

Bracing:

Common issues include:

- corrosion and loosening of bracing connections

Maintenance items include:

- inspection, cleaning, and recoating
- tightening or replacement of fixings

Cladding detail identifying features:

Floor and roof zones covered externally with light or heavy cladding panels (at the discretion of the architect or contractor). Cladding also installed to facades without windows.

Light cladding - consists of tiles, light timber framework with external horizontal boarding, aluminium panelling or similar.

Tiles - typically fixed to 2" × 1½" (50mm × 38mm) timber battens, spaced at a maximum of 4½" (115mm) centres. A 4lb lead flashing is generally installed at the head of the uppermost tile course, with the head cill detail running continuously across the cladding to ensure effective weather protection.

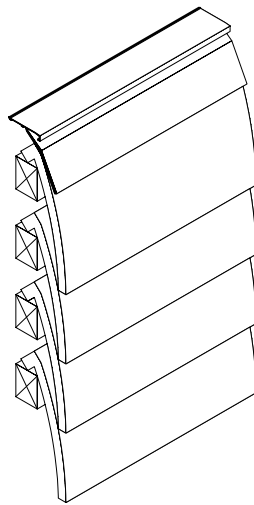


Figure 22: Typical tiled cladding detail used in a SCOLA building

Boarding - shiplap boarding fixed to secondary cladding angle rails, which are in turn secured to the primary structural frame. A 4lb lead flashing is dressed beneath the head cill at the uppermost cladding board location.

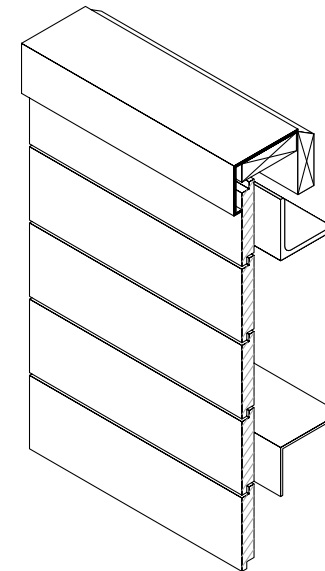


Figure 23: Typical timber cladding detail used in a SCOLA building

Appendix A

Heavy cladding - concrete panels typically 3½" (89mm) thick overall, ranging from 2' (610mm) to 8' (2438mm) in width. The concrete panels typically have an exposed aggregate finish on the external face, with cast-in fixing points on the back face to take ½" (13mm) bolts.

Behind the units, between the 2" (51mm) extrudes, expanded polystyrene insulation was applied directly to the concrete with adhesive.

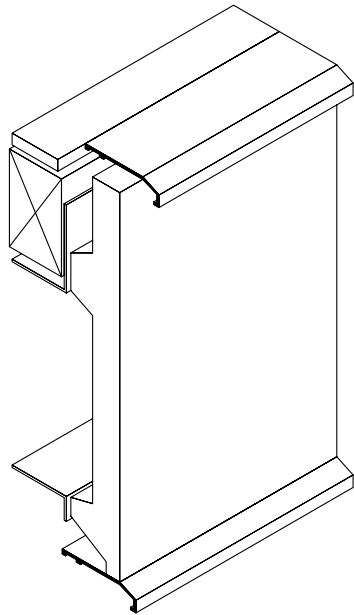


Figure 24: Typical concrete cladding detail used in a SCOLA building

Masonry - 3" (76mm) lightweight concrete block inner skin with dry lining of ½" (13mm) plasterboard on Thistlebond dots, laid in accordance with British Gypsum's recommendations. 1½" (38mm) cavity and 4½" (115mm) brickwork outer skin.

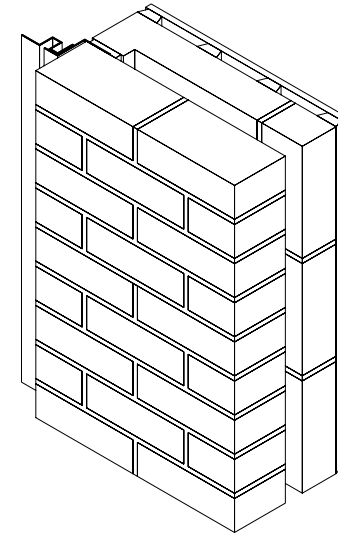


Figure 25: Typical masonry facade detail used in a SCOLA building

Steel frame identifying features:

The structural framework is formed of pin-jointed steel columns, beams and diagonal ties.

The steel beams come in various forms, such as:

Fabricated steel trusses - constructed from back-to-back angle sections for both the top and bottom chords, with solid steel bar diagonals connecting the chords to form a complete triangulated truss system. The angles for the top and bottom chords range from 1½" (38mm) x 1½" (38mm) x ⅛" (3mm) to 4" (102mm) x 2½" (64mm) x 5/16" (8mm). The diagonal steel bars range from ½" (13mm) diameter to 13/16" (21mm) diameter. Spans of the fabricated steel trusses can range from 6' (1829mm) to 30' (9144mm). The fabricated steel trusses are typically complete with a pre-welded horizontal plate below the top chord with 2no. 13/16" (21mm) diameter pre-drilled bolt holes.

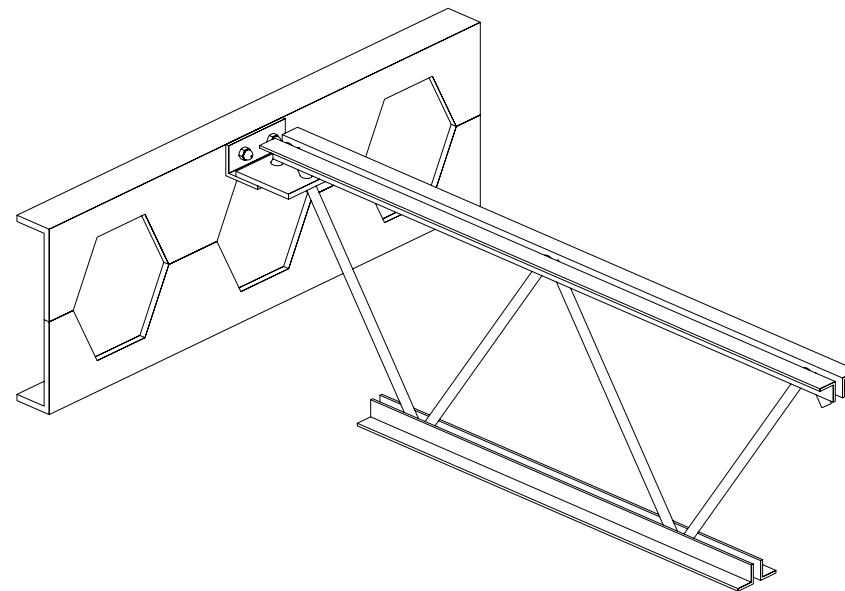


Figure 26: Fabricated steel truss to beam connection in a SCOLA structure

Rolled steel channels (RSC) - typically used for the perimeter beams, range from 3" (76mm) x 1½" (38mm) to 6" (152mm) x 3" (76mm). Spans of the RSC members range from 4' (1220mm) to 12' (3658mm). The end connection detail of the RSCs is typically complete with pre-welded angle sections of 2½" x 1½" x ¼" x 5" long (64mm x 38mm x 6mm x 127mm long) with 1no. 13/16" (21mm) diameter pre-drilled bolt hole.

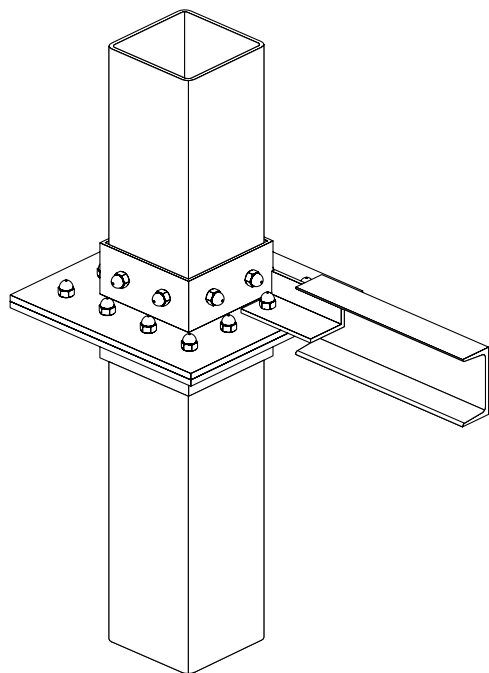


Figure 27: Typical rolled steel channel to column connection in a SCOLA structure

Fabricated channel members - range from 5" (127mm) x 2½" (64mm) to 10" (254mm) x 3" (76mm). Spans of the fabricated channel members range from 4' (1220mm) to 10' (3048mm). The end connection detail of fabricated channel members is typically complete with pre-welded angle sections of 3" x 2½" x 5/16" x 5½" long (76mm x 64mm x 8mm x 140mm long) with 2no. 13/16" (21mm) diameter pre-drilled bolt holes at 31/14" (82mm) centres. Any beam that supports an incoming beam should have angles bolted to the web at the incoming beam location. These angles are typically 3" x 2½" x 5/16" x 5½" long (76mm x 64mm x 8mm x 140mm long).

'LITZKA' channel section - is a fabricated steel member derived from the same component arrangement as the 10" x 3" fabricated channel member but formed to a greater overall depth (up to approximately 14"). The web is modified with regularly spaced hexagonal openings (castellations), intended to create a lightweight, perforated section that enhanced structural efficiency while keeping material weight to a minimum. The 'LITZKA' channel section can span up to 12'. The end connection detail of the 'LITZKA' channel section is typically complete with pre-welded angle sections of 3" x 2½" x 5/16" x 5½" long (76mm x 64mm x 8mm x 140mm long) with 2no. 13/16" (21mm) diameter pre-drilled bolt holes at 3¼" (82mm) centres. Any beam that supports an incoming beam should have angles bolted to the web at the incoming beam location. The angles are typically 3" x 2½" x 5/16" x 5½" long (76mm x 64mm x 8mm x 140mm long).

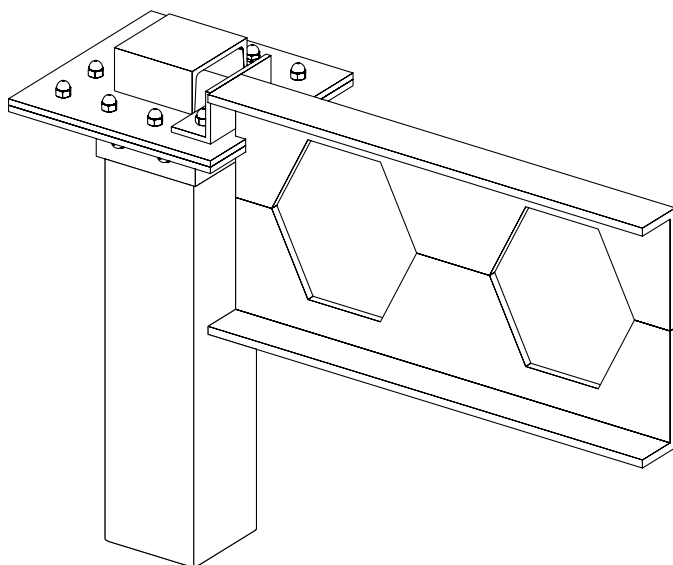


Figure 28: LITZKA beam to column connection in a SCOLA structure

Steel columns - (or stanchions) typically consist of 6" x 6" hot rolled square hollow sections as well as a lighter weight cold rolled section made up of 2no. 6" x 3" x 12 Gauge (2.6mm) cold rolled channels. Column spans range from 9' 11 3/8" (3032mm) to 20' 11 3/8" (6385mm). The columns are typically complete with sockets at the cap (top), intermediate (floor level), and at the base.

The cap sockets are made up of 13" x 13" x 3/8" (330mm x 330mm x 9.50mm) plates with 12no. 13/16" (21mm) diameter pre-drilled bolt holes. The external collar element is made up of 3" x 3/16" (76mm x 5mm) flat plate which has 8no. 13/16" (21mm) diameter pre-drilled bolt holes to allow the collar to be secured to the column faces. The cap is then complete with a 5" x 2 1/2" x 5" long channel section pre-welded centrally to the top of the plate.

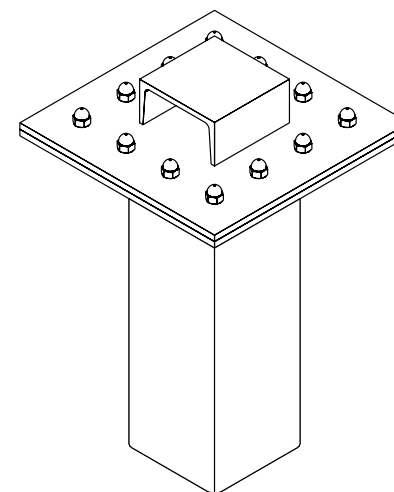


Figure 29: Typical column cap socket in a SCOLA structure

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The intermediate sockets at floor level are made up of 13" x 13" x 3/8" (330mm x 330mm x 9.50mm) plates with 12no. 13/16" (21mm) diameter pre-drilled bolt holes. The external collar element is made up of 3" x 3/16" (76mm x 5mm) flat plate which has 20no. 13/16" (21mm) diameter pre-drilled bolt holes to allow the collar to be secured to the column faces.

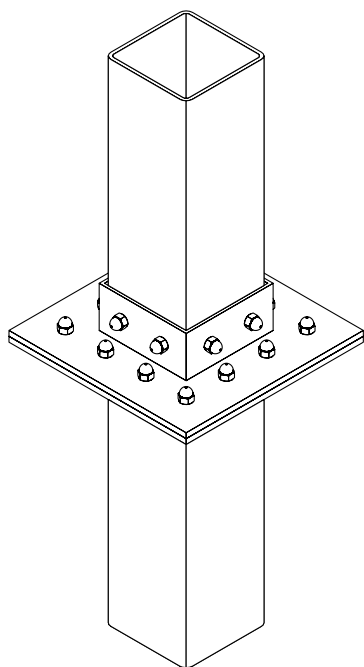


Figure 30: Typical column intermediate socket in a SCOLA structure

The base sockets at floor level are made up of a 9" x 9" x 1/2" (230mm x 230mm x 13mm) plates with a single 1 1/2" (38mm) diameter hole in the centre, set onto a base of grout. The base plate is fixed down with a single holding down bolt of 9" x 1" square-round-square with square neck and a 5" x 5" x 3/8" washer plate secured into the concrete. The holding down bolt typically has 6" embedment. The collar of the base socket is made up of 3" x 3/16" (76mm x 5mm) flat plates pre-drilled with 8no. 13/16" (21mm) diameter pre-drilled bolt holes to allow the collar to be secured to the column faces. The base connections were designed to resist permissible uplifts of up to 4.65 tons (45kN).

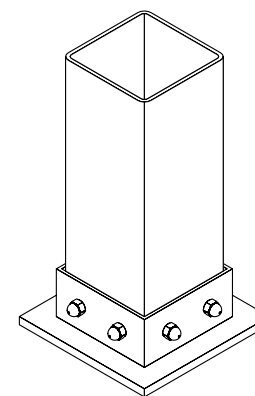


Figure 31: Typical column base in a SCOLA structure

Bracing identifying feature:

Flat steel bracing comprising 2" (51mm) × ¼" (6mm) plates fitted with 3" (76mm) × ½" (13mm) end plates, each provided with a single bolt hole to accommodate a 1" diameter high strength friction grip (HSFG bolt). The flat plates typically bear directly against one another at their intersection, forming the central connection within the braced bay.

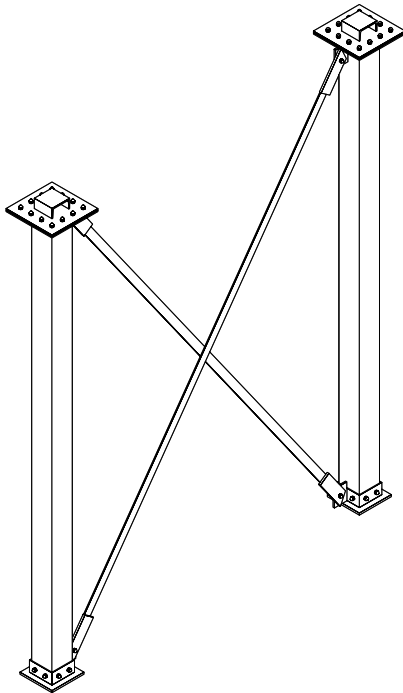


Figure 32: Typical vertical bracing detail in a SCOLA structure

References and further information

- [Non-Industrial buildings – newsteelconstruction.com](http://newsteelconstruction.com)
- [Kendall Kingscott | Retrofit: Transforming School Buildings into Energy-Efficient, High-Performance Spaces](#)

Appendix B: Guide to common construction materials in school buildings

Introduction

The following pages summarise some of the common issues that occur in construction materials which are commonly used in the education estate. For each issue there is a short summary of the common signs of deterioration and actions that should be taken in the short term.

In all cases, further advice should be sought from a suitably qualified and experienced building professional, for example a chartered engineer, architect or chartered building surveyor.

Timber

Key maintenance issue

Overloading

Common signs of deterioration

- excessive deflection of timber floor or roof structure
- cracking or splitting of timber floor or roof structure

Description

Timber structures are lightweight and may not have been designed for storage of heavy materials (e.g., paper, furniture etc).

Short-term action

Ensure that areas where heavy objects are being stored have been designed for the weight of the materials concerned. When planning modifications (e.g., building services replacement) ensure that additional weight on the structure has been allowed for. In particular, check for damage at connections between timber elements.

Key maintenance issue

Wet or dry rot (fungal deterioration)

Common signs of deterioration

- staining, discolouration or softness in timber elements
- splitting or bulging of timber elements
- distortion of timber elements

Description

Wet and dry rot can reduce the load carrying capacity of timber elements. Timber should generally be kept at a moisture content of less than 20%.

Short-term action

For localised damage use commonly available wood fillers to repair damage. Ensure that all damaged timber is removed prior to filling and follow manufacturer's instructions. The cause of the moisture ingress should also be investigated, and roof/wall coverings repaired as necessary. All damp areas should be ventilated so they dry out.

Further advice

Timber specialists can survey timber elements to confirm moisture content. When damage is extensive, consult a suitably

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qualified and experienced chartered engineer who will be able to develop appropriate remedial solutions.

Key maintenance issue

Mechanical damage

Common signs of deterioration

- notching or drilling of timber, particularly near supports

Description

Holes formed in timber elements, particularly near supports, to allow the installation of electrical or heating services etc.

Short-term action

For localised damage, strengthen sections using additional timber, screws or steel brackets.

Concrete - General

Key maintenance issue

Corrosion of steel reinforcement within concrete elements

Common signs of deterioration

- cracking or spalling of concrete (possibly revealing corroded steel reinforcement)
- brown staining to surface of concrete elements
- workmanship issues from poorly installed formwork

Description

Water or chemical ingress into concrete elements can result in steel reinforcement rusting and reducing the load bearing capacity of elements. The corroded steel will expand and in extreme cases will result in some parts of the concrete element breaking off (term 'spalling').

There are many reasons why water can penetrate into concrete elements and cause corrosion. These include where the constituent materials have reacted with the environment, meaning the concrete no longer protects the reinforcement from moisture, or there being insufficient concrete coverage around the reinforcement (this is particularly common for buildings built before the 1980s).

There may also be cases where the concrete was not adequately specified to suit the surrounding environment, for example in coastal areas where the marine environment is more aggressive.

Short-term action

For water staining and very minor hairline cracks between infill masonry walls and concrete structures, identify any water leaks and repair the waterproofing or cladding/pointing locally. Allow to dry-out and monitor regularly where the concrete does not exhibit damage.

Further advice

For minor, localised cracking and/or loss of concrete to secondary elements items such as short span lintels supporting masonry, engage a reputable concrete repair specialist immediately to assess and recommend repairs or other courses of action.

Where damage is more extensive (including cracks wider than 0.5mm width, stepped cracks, random hairline cracking (or 'crazing'), or heavy or unusual loads in the vicinity), a suitably qualified and experienced chartered engineer should be consulted who will be able to advise on the cause of the corrosion. They may specify tests by materials' specialists to confirm their initial findings.

Concrete - Clinker aggregate

Key maintenance issue

Moisture ingress resulting in reinforcement corrosion or material deterioration.

Common signs of deterioration

- cracking or spalling of concrete (possibly revealing corroded steel reinforcement)
- water staining
- brown staining to surface of concrete elements due to rust
- softened material that can be indented with a screwdriver

Description

Clinker is a waste material from manufacturing processes such as cement production, burning of fossil fuels and metal smelting. It was commonly used as a low-cost aggregate for concrete blocks and planks from the 1900s and was still in use in the 1960s. Clinker products contain sulphur and other materials which can be aggressive to steelwork if moisture ingress occurs.

Short-term action

If clinker concrete products have been affected by water ingress, the condition of the concrete should be checked. For water staining and very minor damage, identify and repair any water leaks and waterproofing.

Key maintenance issue

Loss of strength due to mechanical damage or alterations.

Common signs of deterioration

- holes or notches to planks or blocks
- openings or cut-outs for services or rooflights

Description

Clinker concrete products were typically made in factories to standard dimensions and installed on site as floor systems for steel or concrete building frames. Due to the brittle nature of the material, holes or damage to the product could occur during transportation or installation. Holes or openings may also have been formed to allow for arrangements on site including the installation of building services etc which reduces structural integrity.

Further advice

Engage a suitably qualified and experienced chartered engineer when maintenance issues with clinker concrete are identified as a structural assessment should be carried out.

Concrete - High Alumina Cement (HAC)

Key maintenance issue

Loss of integrity due to moisture ingress resulting in reinforcement corrosion or concrete deterioration.

Common signs of deterioration

- concrete being a darker grey colour than surrounding elements, particularly in precast concrete frames
- cracking or spalling of concrete (possibly revealing corroded steel reinforcement)
- water staining
- bowing, cracking of 'bounciness' of floors

Description

High Alumina Cement concrete was popular for precast concrete construction from the 1950s for the way it rapidly gained strength and could provide resistance to aggressive environmental conditions. Over time, these products were found to undergo a chemical conversion where the original strength of the material is reduced and turns the concrete a darker grey colour. The conversion also leaves HAC elements susceptible to further deterioration when subject to water ingress and this can result in concrete cracking or spalling or corrosion to reinforcement.

Short-term action

If you have precast floor or roof beams dating from the 1950s, 60s or early 70s you should get advice on the likelihood of HAC being present from a qualified building professional as part of regular condition monitoring and maintenance.

Further advice

When there is water staining, deflection, minor concrete damage or evidence of corroded reinforcement, consult a suitably qualified and experienced chartered engineer who will be able to undertake investigations to assess the building. Where appropriate, the engineer can develop a suitable management regime and remedial solutions.

References

The following guidance has been published on HAC. There documents are behind a paywall but should be accessible to building professionals who can provide more advice:

- Historical defects in buildings No. 2: High-alumina cement. The Structural Engineer, Volume 101, Issue 11, 2023, Page(s) 24-25
- BRE Special Digest 3: HAC concrete in the UK: assessment, durability management, maintenance and refurbishment. 2002

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- BRE Report BR451: High alumina cement concrete: BRAC rules. 2002

Steelwork

Key maintenance issue

Steel corrosion (rusting)

Common signs of deterioration

- rust staining to internal and external finishes
- damage to external or internal finishes near structural elements
- “bubbly” appearance to steelwork elements, including flaking of paint on steelwork

Description

Steelwork is prone to corrosion when it is in contact with water and high humidity. Water and air react with steel to form iron oxide (rust). The corrosion process is expansive and corroded steelwork can expand by 5 to 10 times compared with the volume of structural steel before corrosion. As a result, the corrosion may cause more damage to surrounding elements as it expands than to the structural steelwork itself. Areas such as plant rooms and external facades are therefore particularly vulnerable.

Short-term action

Although non-reversible, the effects of corrosion can be minor for the steelwork if caught early. While unsightly, corrosion of steelwork is expansive meaning large areas of rust can be caused by minor areas of corrosion. Affected areas should be cleared with a wire brush to remove all loose material and to reveal clean (shiny) steel. Readily available repair and protection products can then be applied to the affected area. The source of water penetration should also be identified and repaired to avoid recurring issues.

Signs of corrosion around bolted steel connections should be investigated. If the deterioration is excessive, there is the risk of sudden collapse.

Further advice

Engage a suitably qualified and experienced chartered engineer when corrosion is more extensive as an assessment will need to be made on the loss of capacity due to the corrosion process.

Masonry

Key maintenance issue

Damage to pointing

Common signs of deterioration

- erosion/recessing of masonry pointing

Description

Pointing is the finishing of mortar joints between layers (or 'courses') of masonry. The mortar may be softer than the surrounding brick and may be damaged by wind-blown rainwater. Areas of damaged pointing can result in water ingress and in extreme cases can affect the load carrying capacity of the masonry walls.

Short-term action

Affected areas should be removed, joints between brickwork cleaned and a suitable mortar used to repoint the affected areas. It is important that the correct mortar is used for repointing otherwise further damage could be caused.

Further advice

Further advice can be obtained from a brickwork expert or stonemason, chartered building surveyor or architect.

Key maintenance issue

Loss or damage of cavity ties

Common signs of deterioration

- 'bulging' of walls or walls being out-of-plumb (not vertical)
- parallel horizontal cracking at joints
- vertical constant width cracking adjacent to corners

Description

Early masonry ties did not always have sufficient corrosion protection applied and may be prone to rust. This corrosion is expansive and can 'push' the masonry construction upwards. In severe cases, ties completely corrode leading to instability of the masonry construction, noticeable through an outward bulge.

Similar defects are noticeable when there are insufficient ties between the inner and outer leaves.

Short-term action

Monitor cracks regularly to see if they continue to expand. Install crack monitors, also known as 'tell tales' which will confirm whether movement is progressive.

Key maintenance issue

Foundation movement

Common signs of deterioration

- cracking in masonry walls
- distortion of masonry elements

Description

Foundation movement can be caused when the ground moves underneath a building. This is particularly noticeable on masonry buildings as they are brittle (i.e., crack easily) and are a relatively heavy construction type. Common causes of foundation movement are softening of the ground due to nearby cracked drains, ground movement due to water demand from nearby trees, or expansion and contraction of the ground due to hot or cold weather (particularly in clay soils). Foundation movement is common and only needs remedial work if it is progressive (i.e., if movement continues to occur).

Short-term action

Monitor cracks regularly to see if they continue to expand. Install ‘tell tales’ which will confirm whether movement is progressive.

Key maintenance issue

Chimney and parapet instability

Common signs of deterioration

- chimneys and parapet walls being out-of-plumb (not vertical)

Description

Chimneys and parapets can be damaged by any of the previously mentioned issues. Because they are slender, and often in exposed locations, they can be particularly susceptible to more extreme damage than load bearing masonry walls.

Short-term action

Regularly inspect chimneys and parapets for signs of movement. Monitor areas of concern closely, particularly after extreme weather. If movement is continual (progressive), consider creating exclusion areas to avoid damage from falling debris.

Further advice

Seek advice from a chartered building surveyor or structural engineer. In extreme cases, contact your [local council building control team](#) who can advise on dangerous structures.

Appendix C: Guide to common construction types in school buildings

Introduction

The following pages summarise some of the common issues that occur with common construction in the education estate. For each issue there is a short summary of the common features, signs of deterioration and actions that should be taken in the short term.

Based on the information in this Appendix, the reader should be able to identify if they have buildings with these components and areas that should be given special attention and consideration for targeted inspection and maintenance.

In all cases, further advice should be sought from a suitably qualified and experienced building professional, for example a chartered engineer, architect or chartered building surveyor.

Beam and hollow concrete pot systems



Figure 33: Example of a soffit to a beam and hollow concrete pot floor system

Background

The term 'beam and hollow concrete pot' flooring is a subset of 'beam and block' construction, which has been used since the early to mid-20th century.

'Beam and block' construction includes:

- **beam and solid block** construction comprising precast concrete beams spaced at 300mm to 600mm centres with solid concrete blocks infilling the gaps between the beams. They are commonly used in house building today
- **beam and hollow clay pot** construction comprising hollow clay bricks (pots) laid on formwork with wet concrete cast on top and around them. These were commonly used in the early to mid-20th century
- **beam and hollow concrete pot** construction using factory made components as precast concrete x-beams, joists or 'ribs'. These were commonly used in post-war buildings

Beam and hollow concrete pot floor systems are typically constructed from precast concrete beams or 'ribs' at regular spacings with hollow concrete blocks or 'pots' placed on site to infill between the beams. A concrete topping or screed may also be laid over the beams and pots to complete the floor structure. These floor and roof systems have less steel reinforcement in them than would be expected in modern buildings.

In the post-war period, a range of manufacturers including Fram Precast Concrete Ltd and Head Wrightson Teesdale Ltd manufactured 'ribs' and 'pots' of different shapes and sizes. The

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properties and load carrying capability varies between systems. Figures 35 and 36 overleaf show key identifying features.

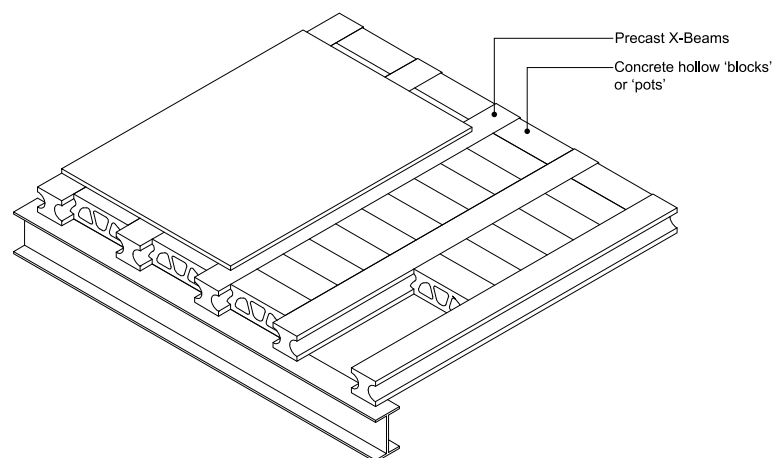


Figure 34: Example of a beam and hollow concrete pot floor system showing the repeating structural elements

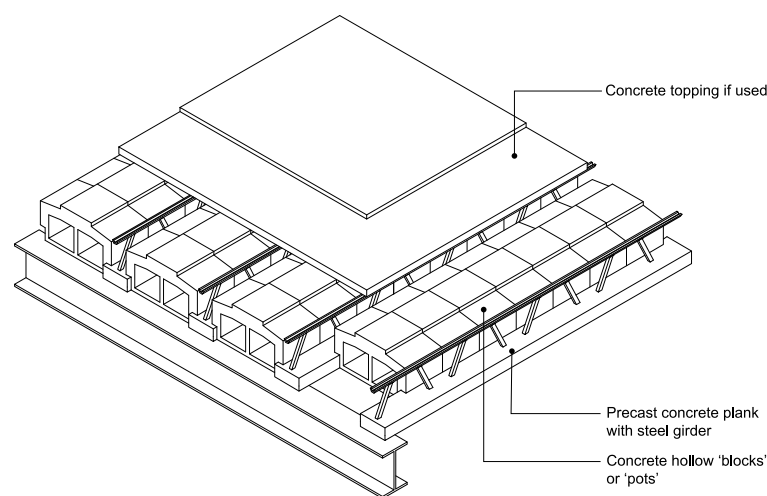


Figure 35: Example of a precast concrete X-beam floor system showing the ribbed structural elements

Common signs of deterioration

Cracks in hollow concrete pots caused by:

- defects from the time of installation
- settlement or thermal movement of a building
- historic modifications e.g. fixing building services
- changes in loading (e.g. changes in room use, concentrated loads directly over hollow pots, new ballast when photovoltaic panels have been installed)
- deterioration of HAC beams or 'ribs' (see Appendix B)

Cracking of individual pots is unlikely to cause instability of a building. It is unlikely that cracking in a pot will weaken or overstress adjacent pots.

Further advice

No new fixings should be made into the concrete pots as they may be brittle and could be damaged. If existing fixings have become loose, they should be relocated to the concrete ribs or bridging structure should be provided.

Beam and hollow concrete pot floors and roofs should regularly be inspected for cracking. Inspections should be more frequent when part of the building has changed use, been modified or structural movement has been noticed elsewhere. If cracking or other deterioration is observed, engage a suitably qualified and experienced chartered engineer to advise on whether any mitigation is required.

Proprietary timber 'ply web' components

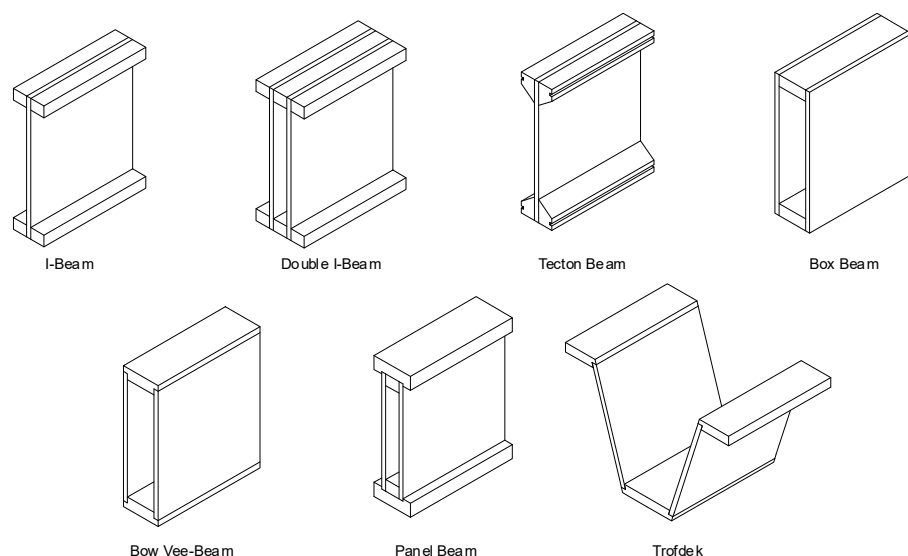


Figure 36: Examples of timber ply web beams used in floor or roof construction

Background

Timber 'ply web' is a general term for sections of softwood which are pinned and glued to plywood sheets to form either discrete timber beams or timber floor cassette systems (e.g., 'Trofdek' - see Figure 38). These components are commonly found in post-war construction and were a predecessor to engineered timber joists used in construction today. They can span further than sawn timber joists, making them suitable for halls and assembly spaces in schools.

Prefabricated folded plate timber roofs are constructed by structurally nailing or gluing timber and plywood sections together to form 'troughed' beams. These beams were typically overlaid with plywood sheeting and supported by an end board with timber battens glued or nailed to the plywood 'trough' of the beam. Where abutting Trofdek beams are used, a 'splice' joint is formed in the timber and plywood sections. One system was patented as 'Trofdek' in North America and the UK and was supplied to several counties by H Newsum Sons and Co Ltd (as known as Newsum Timber Engineers Ltd) in the 1950s and 1960s.

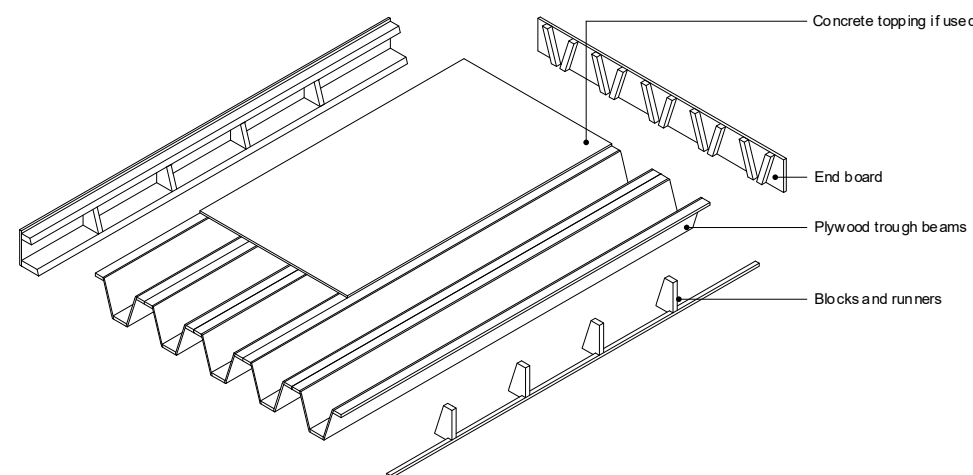


Figure 37: Example of a folded plate timber roof constructed using the Trofdek system

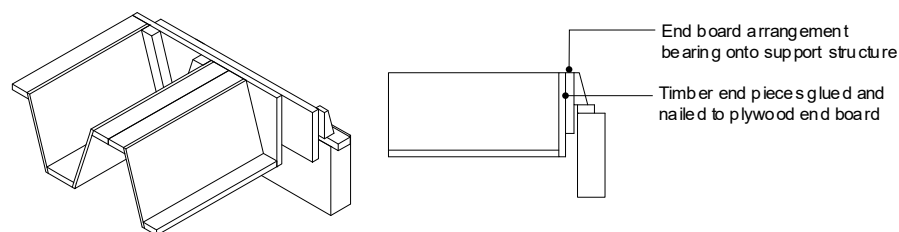


Figure 38: Example of a bearing support detail for a Trofdek folded plate timber roof

Trofdek units are identified by:

- angled plywood 'webs' each side of a timber baseboard to form a 'trough' beam
- plywood sheeting to top of beams
- end board at bearing supports
- double timber boards at the top of 'troughs' to support joints along the beams
- 'trough' beams can be identified by removing ceilings, inspections at existing openings for rooflights or service voids or by drilling a small hole in the soffit board for a borescope inspection. Any investigation work should be carried out carefully and in accordance with asbestos guidance to avoid disturbing asbestos containing materials

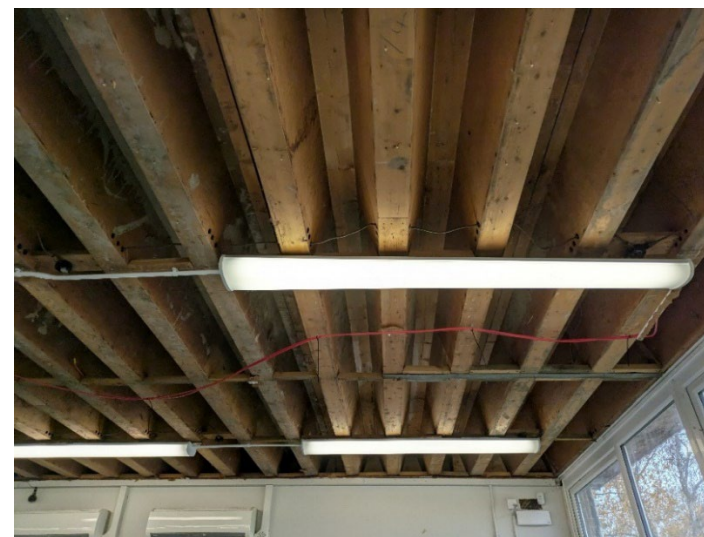


Figure 39: Example of a soffit of a Trofdek roof

Common signs of deterioration

- building movement causing the bearing length at supports to be reduced
- building movement at splice joints of abutting 'trough' beams due lack of fasteners and timber reinforcement
- water ingress or moisture damage which may cause deterioration of the timber near glued joints
- sagging or deformation of ceiling panels
- excessive deflection to 'trough' beams
- blocked rainwater outlets and gulleys causing water to pond on roof which may overload the structure or increase the risk of water ingress

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Further advice

Investigate any areas of roof ponding or water ingress and ensure that gutters and rainwater goods are clear to allow water to drain from roofs. If there are signs of water ingress, deformation to ceilings or excessive deflection, engage a suitably qualified and experienced chartered engineer. The chartered engineer should also inspect the bearings and 'splice' joints of 'ply web' systems as part of routine maintenance.

Consideration should also be given to the replacement of fixed ceiling boards with a suspended ceiling tile system to allow for regular monitoring of the Trofdek structure as part of a building's ongoing maintenance plan.

References and further information

- [Cross UK: Collaborative Reporting for Safer Structures UK](#)
- The Construction of Buildings, Volume 3: Barry R, 1963, p67-69
- An Introduction to Timber Engineering: Andrews H, 1967, p77-78
- Proprietary Roof and Beam Systems, No 12 'Trofdek' System: Supplement to Wood, July 1965
- Timber Designers' Manual: Ozelton and Baird, 1976, Chapter 8

Woodwool / Stramit



Figure 40: Woodwool roof slab supported by steel trusses

Background

These panels were commonly used in roof construction in the 1960s and consist of compressed timber/sawdust (woodwool) or straw/paper (Stramit). They were generally used to support and insulate concrete and were sometimes used as the roof structure (or 'deck') itself with insulation and felt applied directly to the woodwool or Stramit.

Due to their composition, they can rapidly lose strength if subject to water ingress leading to sagging and failure.

Key maintenance issue

Loss of strength due to water penetration

Common signs of deterioration

- water staining
- sagging of panels
- decomposition of woodwool or Stramit panels

Short-term action

Where woodwool or Stramit has been affected by water ingress, consideration should be given to risk assess and manage the use of the affected spaces.

Further advice

Consideration should be given to the replacement of fixed ceiling boards with a suspended ceiling tile system to allow for regular monitoring of Woodwool or Stramit panels as part of a building's ongoing maintenance plan.

Consideration should also be given to the replacement of woodwool and Stramit as part of a building's ongoing maintenance and refurbishment plan.

Lath & plaster

Background

Lath and plaster was used between the 18th century and the early to mid-20th century. It comprises thin strips of wood (laths) approximately 25mm wide, fixed together at very close spacing.

It provides a flat surface for painting and is unlikely to form part of the original load bearing structure of a building. It was replaced with the advent of gypsum plaster and sheets of plasterboard. Depending on a building's age it may be providing some additional support, particularly to limit horizontal movement.

Laths are typically fixed to timber joists which may be part of the load bearing structure. Coats of plaster were applied to the laths which, when dried, provided a flat finished surface.

Key maintenance issue

Lath and plaster is highly absorbent. If exposed to water, it can become heavy, lose its strength or become detached from its fixings.

Common signs of deterioration

- water staining
- cracking of paintwork or pieces of plaster becoming detached

- bulging or warping of wall and ceiling finishes
- plaster becoming rough

Short-term action

Lath and plaster walls and ceilings require no additional maintenance than other internal wall and ceilings linings, provided they are kept dry and inspected regularly.

If new cracks are noticed in lath and plaster ceilings, they may be an indication of recent movement. Small areas of lath and plaster can be removed by a builder to allow the structure behind to be inspected.

Further advice

If there is suspicion of water ingress into areas with lath and plaster, these should be investigated thoroughly and damaged gutters, downpipes or waterproof flashings repaired. Once water damaged, the strength of the lath and plaster may be reduced, and replacement should be considered as part of routine maintenance.

Further advice can be sought from a suitable experienced surveyor or structural engineer, particularly in the case where for older buildings or where there is more extensive cracking is present. Building contractors with experience of lath and plaster can advise on any investigations or remedial works.

TonkinDek



Figure 41: Example of a TonkinDek slab roof system constructed from composite board materials

Background

TonkinDek or Tonkin Deck slab materials were used in the Cornwall region as roof construction elements, formed from waste products from local manufacturing processes. The slab elements are consistent with thin high-density cement bound wood particleboards.

Key maintenance issue

Loss of strength due to water ingress resulting in deterioration of TonkinDek slab material.

Common signs of deterioration

- cracking or spalling of slab material (possibly revealing corroded internal reinforcement)
- water staining
- softened material that can be indented with a screwdriver
- sagging of horizontal roof slab elements

Short-term action

If TonkinDek slab materials have been identified, the condition of the material should be checked by a qualified building professional as part of regular condition monitoring and maintenance. Consider improvements to ensure slab materials are kept dry including maintenance of roof coverings, waterproofing detailing and drainage systems. Investigate and remove the build-up of water and debris on roof panels to reduce the risk of overloading.

No new fixings should be made into the TonkinDek material as they could cause damage to the material or waterproofing felt above. If existing fixings have become loose, they should be relocated to the concrete ribs, or bridging structure provided.

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Further advice

Where TonkinDek slab materials have been affected by water ingress or there is noticeable sagging of slab elements, engage a suitably qualified structural engineer to assess and advise on the management of affected areas. Building contractors with experience of TonkinDek slab materials can also advise on appropriate locations for any new apertures, fixtures or cabling through slab elements.

Consideration should also be given to the replacement of TonkinDek slab materials as part of a building's ongoing maintenance plan.

References and further information

- BRE Digest 477: Wood-based panels (7-part set)

Appendix D: Historic design practices

Introduction

The advent of new materials, technologies and learning lessons from past structural collapses has led to structural design practices evolving over time. Codes of practice, guidance and best practice have all developed.

There is no requirement for existing buildings to be brought up to modern building standards, unless significant modification is being carried out, in which case the current requirements of The Building Regulations 2010 must be met. In these cases, a Registered Building Inspector should be appointed who can provide detailed advice.

This section is designed to help those who operate and manage education buildings to consider whether their maintenance and monitoring regime should account for historic design practices, for example through targeting inspections or giving extra consideration when changing how areas are used.

Structural robustness

Background

The principle of structural robustness is used by structural engineers to describe the way a building is designed so that unexpected events (for example, a gas explosion or vehicle impact) do not result in a disproportionate amount of damage to the structure. A building with structural robustness has redundancy (i.e., spare capacity) in its structure to continue supporting loads if one structural element becomes unusable. This limits the spread of damage and prevents other elements becoming overloaded which could cause extensive damage.

The principles of robustness have been included in The Building Regulations since 1970, following the partial collapse of Ronan Point (a 22-storey high rise) in 1968 when a small gas explosion resulted in one side of the tower collapsing. Although buildings designed prior to 1970 may not have had robustness considered explicitly in their design, many have inherent robustness from the way the load bearing structure is arranged and connected, allowing loads to be shared and with alternative paths to the ground if damage occurs. There is no requirement for education buildings constructed prior to 1970 to be upgraded to meet current requirements unless other modifications are being carried out that require sign off by a Registered Building Inspector.

Key considerations

All building owners and operators should take appropriate steps to minimise the likelihood of accidental events which may damage their building. This includes structural deterioration caused by lack of maintenance. This is particularly important for those buildings that have not been designed to meet modern robustness requirements.

Risk management

Managing risks requires detailed knowledge of the building and its surroundings. As such, the guidance here should be considered high-level and further advice should be sought from a qualified professional if the building owner has specific concerns. Annex B of BS EN 1991-1-7 (Eurocode 1 - Actions on structures. Accidental actions) gives detail on how a risk assessment should be carried out. This should begin with a qualitative risk assessment to identify all hazards and hazard scenarios. This is known as hazard identification (HAZID). Some examples of hazards include (but are not limited to):

- overloading, either accidental or by mismanagement (see [Floor and roof loading](#))
- deterioration of the structure (for example, due to lack of maintenance)
- changes to weather or ground conditions

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- accidental events (e.g., explosion, fire, flood, vehicle impact)

When defining a hazard, the following should also be considered:

- the building's use
- the building's surroundings
- the inspection and monitoring regime for the building
- the construction type and materials in the building, including any areas susceptible to damage
- the consequence of any damage

The probability and consequence of each of the risks should be considered when deciding the next steps to be undertaken.

Further actions should either:

- eliminate (i.e., remove) the hazard. An example of this would be alternative arrangements for vehicle movement to avoid the risk of vehicle impact
- protect against the hazard. An example of this would be ensuring that all necessary certification was in place for gas systems to confirm they are safe (which should already be in place)
- control the hazard. An example of this would be increased monitoring of structural elements where deterioration is noticeable
- remediate or strengthen if the above cannot be reliably carried out

Risk management works on the basis of risks being 'as low as reasonably practicable' (ALARP). This means that all mitigation should be proportionate and put in place at the earliest opportunity. The time, resources and financial implications of any mitigation should be balanced against the benefit(s).

Further advice

Engage a suitably qualified chartered engineer to review building information and undertake an assessment where there are concerns regarding robustness. The engineer should have relevant experience in the assessment of older buildings and the various unusual materials and arrangements that may have been used.

References and further information

- Structural robustness and disproportionate collapse in buildings (Second edition), The Institution of Structural Engineers, 2023
 - BRE: Handbook for the structural assessment of large panel system (LPS) dwelling blocks for accidental loading, S Mathews and B Reeves, 2012
 - Preparing safety case reports for higher-risk buildings under the Building Safety Act: guidance for structural engineers, The Institution of Structural Engineers, 2026
- Note:** This is not directly applicable for education settings which are not defined as 'high risk buildings', but its explanation of risk assessment applies to all structures

Floor and roof loading

The weight of construction materials, fixtures and fittings, and occupants is a key consideration in the structural design of a building. For a structure perform as required, these forces, along with those from the external environment (for example, wind, snow and thermal effects) must be transferred to the ground without the structure being in distress. The forces which a building should be designed for are set out in guidance and codes of practice.

This section deals only with 'live' loads, those applied by occupants, fixtures and fittings and which require day-to-day management. 'Dead' loads, those applied by building materials (including floor and roof finishes) are likely only to change during significant refurbishment which should be designed and carried out by competent professionals and tradespeople.

Figures 42 to 45 show the design loadings set out by current codes of practice for common uses of education buildings (specifically the variable actions or 'live' loads stated in the National Annex to BS EN 1991-1-1). When considering changes of use to spaces (for example, from a classroom to a library or storage area) it is important to consider whether the loads applied in the new configuration may exceed those considered in the original design. If this is the case, the structure may be overloaded resulting in damage or collapse.

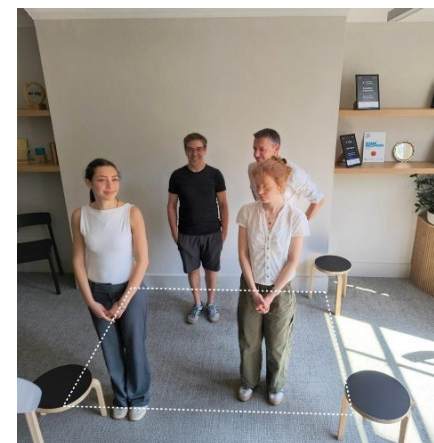


Figure 42: Image showing permissible loadings for a roof equating to 0.6kN/m² (equivalent to 60kg/m²)

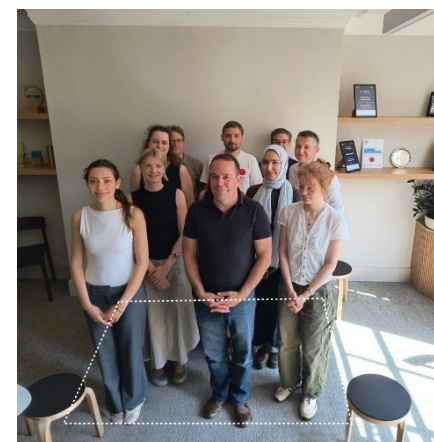


Figure 43: Image showing permissible loadings for a roof equating to 1.5kN/m² (equivalent to 150kg/m²)



Figure 44: Image showing permissible loadings for a classroom equating to 3.0kN/m² (equivalent to 300kg/m²)



Figure 45: Image showing permissible loadings for a corridor or stair equating to 4.0kN/m² (equivalent to 400kg/m²)

Installation of photovoltaics (PVs) on existing roofs

The installation of PV arrays on roofs can reduce energy costs and associated carbon emissions. You should work with a PV specialist to determine the best design, considering how the panels can be orientated and tilted based on the design of your roof.

At the earliest available opportunity, a suitably qualified and experienced chartered engineer should assess the load carrying capacity of the roof to confirm it will not be overloaded by either the weight of the PVs themselves or the effects of additional wind loads. They should also consider how the PVs will be fixed to the roof and any other changes in the use of the roof (for example, new walkways and handrails required for additional maintenance access). If the new installation does overload the existing structure, strengthening or other modifications may also be required.

It is not considered appropriate to use the clauses of Approved Document A on 'Re-covering of roofs' (Section 4) to justify the additional loads.

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Further advice

When considering changing the use of a space you should consult record information showing the loads the structure was originally designed for. If this is not available, you should seek advice from a chartered structural engineer. They may carry out a desk study of historic design guidance. In some cases, they may recommend intrusive investigations to understand more about the structure and its materials. These are likely to be costly, so consideration should be given as to whether the benefits of the change in use justify the cost.

Image credits

Figure 1: Department for Education

Figure 2: Jennifer Singer

Figure 3: Department for Education

Figures 4 & 5: Andrew Rolf

Figure 6: Reds10

Figure 7: Bowmer + Kirkland

Figures 8 to 41 (inclusive): Department for Education

Figures 42 to 45 (inclusive): Useful Simple Trust



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