

SM-ALR Monitoring

M25 J23-27 Second Year Evaluation Report Highways England

March 2017



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Table of contents

| Cha | apter | Pages |
|-------------|---|-----------|
| Exec | cutive summary | 5 |
| 1. | Introduction | 7 |
| 1.1. | Scope of project and purpose of this report | 7 |
| 1.2. | Background of the scheme | 7 |
| 1.3. | Evaluation timescales | 8 |
| 1.4. | Expected effects of SM-ALR | 9 |
| 2. | Flows | 14 |
| 2.1. | Introduction | 14 |
| 2.2. | Daily flows per link | 14 |
| 2.3. | Flow over each time slice per link | 18 |
| 2.4. | Tests for statistical significance of the results | 21 |
| 2.5. | Summary | 22 |
| 3. | Journey times | 23 |
| 3.1. | Introduction | 23 |
| 3.2. | Average journey time | 23 |
| 3.3. | Journey time reliability | 25 |
| 3.4. | Summary | 25 |
| 4. | Safety | 27 |
| 4.1. | Introduction | 27 |
| 4.2. | Number and rate of collisions | 27 |
| 4.3. | Casualties, FWI and KSI rate | 29 |
| 4.4. | Additional analysis | 30 |
| 4.5. | Summary | 32 |
| 5. | Conclusions | 33 |
| 5.1. | Flow | 33 |
| 5.2. | Journey time | 33 |
| 5.3. | Safety | 33 |
| App | endices | 34 |
| Appe | endix A. Flows additional information | 35 |
| A.1. | 24 hour average daily traffic (ADT) | 35 |
| A.2. | Flows by time slice | 36 |
| Appe | endix B. Journey times additional information | 37 |
| B.1. | Days in sample | 37 |
| B.2. | Journey time | 37 |
| Appe | endix C. Safety additional information | 39 |
| C.1. | Contributory factors | 39 |
| C.2. | Red X compliance | 45 |

Tables

| Day type and time slice definitions | 9 |
|--|---|
| Flow by time slice t-tests | 21 |
| ADT t-tests | 22 |
| Journey time metrics | 25 |
| Number of collisions by severity and collision rates | 27 |
| Number of collisions and collision rates following national trends | 29 |
| Number of casualties and FWI rate | 29 |
| Total KSI and KSI rate | 30 |
| Summary of ERA activity | 31 |
| Vehicle types using ERAs | 32 |
| Summary of Red X events | 45 |
| | Day type and time slice definitions Flow by time slice t-tests ADT t-tests Journey time metrics Number of collisions by severity and collision rates Number of collisions and collision rates following national trends Number of casualties and FWI rate Total KSI and KSI rate Summary of ERA activity Vehicle types using ERAs Summary of Red X events |

Figures

| Figure 1-1 | Geographical location of the M25 J23 to J27 SM-ALR scheme | 7 |
|-------------|---|----|
| Figure 1-2 | M25 J23 to J27 SM-ALR scheme split | 8 |
| Figure 1-3 | Data collection & evaluation periods | 9 |
| Figure 1-4 | Snapshot of speeds by lane Before and Yr2 After | 10 |
| Figure 1-5 | Flow by lane Before and Yr2 After | 11 |
| Figure 1-6 | Speed distribution Before and Yr 2 After | 12 |
| Figure 1-7 | Speed flow curves Before and Yr2 After | 13 |
| Figure 2-1 | Average daily traffic by day type J23-J24 clockwise | 15 |
| Figure 2-2 | Average daily traffic by day type J24-J25 clockwise | 15 |
| Figure 2-3 | Average daily traffic by day type J25-J26 clockwise | 16 |
| Figure 2-4 | Average daily traffic by day type J23-J24 anticlockwise | 16 |
| Figure 2-5 | Average daily traffic by day type J24-J25 anticlockwise | 17 |
| Figure 2-6 | Average daily traffic by day type J25-J26 anticlockwise | 17 |
| Figure 2-7 | Average flow by time slice J23-J24 clockwise | 18 |
| Figure 2-8 | Average flow by time slice J24-J25 clockwise | 19 |
| Figure 2-9 | Average flow by time slice J25-J26 clockwise | 19 |
| Figure 2-10 | Average flow by time slice J23-J24 anticlockwise | 20 |
| Figure 2-11 | Average flow by time slice J24-J25 anticlockwise | 20 |
| Figure 2-12 | Average flow by time slice J25-J26 anticlockwise | 21 |
| Figure 3-1 | Clockwise journey time comparison | 23 |
| Figure 3-2 | Anticlockwise journey time comparison | 24 |
| Figure 3-3 | Clockwise journey time reliability analysis | 25 |
| Figure 3-4 | Anticlockwise journey time reliability analysis | 26 |
| Figure 4-1 | Example lane closure event | 30 |
| | | |

Executive summary

Background

Expanding evidence base to provide ongoing confidence Smart motorways are a technology-driven approach to the use of our motorways. They increase capacity and improve journey time reliability while maintaining safety.

The Smart Motorway All Lane Running (SM-ALR) scheme, M25 J23 to J27, has previously been monitored and evaluated for a one year After period. The evidence base is being continually expanded, providing ongoing confidence in the ALR concept. Atkins was therefore commissioned to perform a wide-ranging, comprehensive evaluation of the second year of operation. It is crucial that the performance of the scheme is accurately assessed in order to:

- review the safety performance during the initial period of operation;
- continue to monitor and understand the change in risk to road users and to road workers;
- quantify and provide evidence of the benefits of the concept; and
- provide evidence to help improve the concept of operation and the design requirements.

This report presents the results following the second year of After evaluation from May 2015 to April 2016. It is split into sections to cover each of the objectives of SM-ALR assessed in this report:



flows;

•

- journey times; and
- safety.

Overview of Year 2 Results compared to Before Period

| M25 J23-J | 27 | |
|--------------------------------|---|-------------------|
| Flows | J23-6: Significant (10%) flow increase achieved and capacity for more growth In particular 17% J24-25 CW. All higher than national trends. | |
| Average journey time | JTs returned close to pre-scheme levels but have been worse if scheme not built. CW 3% increase overall, ACW 0.5% decrease. | \Leftrightarrow |
| Journey time reliability | Slight improvement day-to-day on both carriageways | |
| Safety | No significant change after taking into account background trends. Scheme has met its safety objectives. | \Leftrightarrow |

Flow has increased between 6,000 and 11,000 vehicles per day, significantly above national trends

Clockwise some journey times have increased, but would be worse without the scheme

Anticlockwise journey times are improved

Journey time reliability has improved slightly

Flows

Between J23 and J26, average daily flows have increased in excess of 10% on all links. J24 to J25 has seen 17% increase for clockwise flow and 12% for anticlockwise. All links have increased at a higher rate than national trends. No data was available for J26 to J27.

The results demonstrate that significant capacity improvements have been achieved, supporting efficient movement of goods and services on this key section of the SRN; in addition there is still spare capacity to support future growth.

Journey Times

Clockwise, the average journey time to traverse the scheme has increased by 3% compared to before, which equates to 26 seconds additional journey time per vehicle. Monday to Thursday AM peaks have seen the largest increase of 8%. The journey time increase in the second year of operation is probably related to the increase in flow and appears to be mainly due to congestion at the J25 roundabout; this is currently being upgraded. It should be noted that if the scheme had not been built, journey times would almost certainly have deteriorated further, without the significant extra capacity being achieved.

Anticlockwise there is a 0.5% improvement in journey time to traverse the scheme, which equates to a journey time saving of 5 seconds per vehicle; effectively no change in average journey times. The Monday to Thursday PM peak has the largest journey time saving of 5%. Again, journey times have increased in the second year as the flows have increased and are now close to pre-scheme levels.

There has been a slight improvement day to day in journey time variability in the most congested periods on both carriageways, demonstrating that journey times are now more reliable. Some other periods have seen a slight increase but overall there is a slight improvement in reliability while flows have increased more than 10%.

The scheme has achieved its safety objective

Safety

The collision rate has reduced by 11% overall, representing a 1% increase after taking into account the national trend between periods; this is not statistically significant and should be considered as 'no significant change'. This suggests that the scheme is achieving its objective of maintaining safety performance, although further monitoring is required due to the small sample size.

FWI rate and KSI rate reduced, but these results are based on a small sample size, so no conclusions should be drawn.

No collisions were reported involving road workers on ALR.

Compliance with Red X signals was observed on average to be 96% of the total flow on the carriageway during the lane closure. This is an improvement on 93% in the first year of operation.

82% of ERA stops not an emergency

During ERA monitoring one stop every 2 hours per ERA was observed. Nonemergency use was judged to be 82%. In 3% of all stops the ERT was used and in 7% of all stops a Highways England Traffic Officer attended. Other ERA observations were:

- no instances of problems with ERA operation were observed; and
- no collisions relating to vehicles exiting ERAs.

1. Introduction

1.1. Scope of project and purpose of this report

Having completed the monitoring and evaluation of the first year of operation, Highways England commissioned this project to monitor and evaluate the impact following the second year's operation of the first SM-ALR scheme, M25 Junction 23 to Junction 27. The evidence base is being continually expanded, providing ongoing confidence in the ALR concept. It is crucial that the performance of the scheme is accurately assessed for a second year of operation in order to:

- review the safety performance during the initial period of operation;
- continue to monitor and understand the change in risk to road users and to road workers;
- quantify and provide evidence of the benefits of the concept; and
- provide evidence to help improve the concept of operation and the design requirements.

As part of the previous SM-ALR Monitoring project, an evaluation methodology was designed. The analysis for the Before period and both Yr1 After and Yr2 After follows this methodology to ensure that all results are comparable.

The report is split into sections to cover each of the objectives of SM-ALR: flows, journey times and safety.

1.2. Background of the scheme

1.2.1. Location

The SM-ALR scheme, M25 J23 to J27, is part of the key strategic orbital route around London which forms the hub of the English motorway network; it is also a commuter route for local traffic. It lies within the counties of Hertfordshire, Essex and the Greater London Authority and is located in the northern segment of the M25. J23 is the intersection with the A1(M) and J27 the intersection with the M11.

Figure 1-1 Geographical location of the M25 J23 to J27 SM-ALR scheme



The SM-ALR scheme encompasses two tunnels, Holmesdale located between J25 and J26 and Bell Common between J26 and J27.

The majority of the M25 is Smart Motorway with hard shoulders which, together with the SM-ALR scheme, form an overall long term strategy to manage the existing motorway network more effectively.

1.2.2. The SM-ALR scheme

SM-ALR is a controlled four lane carriageway with no hard shoulder. This is supported by technology in the form of Motorway Incident Detection and Automatic Signalling (MIDAS) traffic detection and traffic control. The signs and signals can be controlled by operators and by automatic algorithms for Congestion Management (CM) and Queue Protection (QP). Emergency Refuge Areas (ERAs) are available for emergencies.

This scheme was opened in two stages, J23 to J25 opening in May 2014 and J25 to J27 opening in November 2014, see Figure 1-2.





1.3. Evaluation timescales

This report presents the results of evaluation and monitoring following two years' operation of the scheme from May 2014 to April 2016. For clarity and efficiency, the evaluation periods will be referred to as follows throughout this report:

- Before Baseline;
- Yr1 After First year after opening;
- Yr2 After Second year after opening; and
- After Period Entire after period.

The evaluation makes comparisons between the Before and After periods, while monitoring has taken place during the After periods only. The monitoring results report compliance with Red X and ERA usage.

Figure 1-3 shows the evaluation periods used for the Before and After periods.

This scheme was opened in two stages, J23 to J25 opening in May 2014 and J25 to J27 opening in November 2014. Consequently, the After safety analysis of STATS19 data is based on the data for J23 to J25 only until November 2014.

The flow and journey time evaluations compare the Before with Yr2 After; the last 6 months of Yr1 After are not comparable due to the different duration so are not included in this evaluation. (The results for Before versus Yr1 After can be found in the First Year Evaluation report.) The monitoring results are presented only for Yr2 After and again, the Yr1 After results can be found in the first year report.





For the analysis of flows and journey times it is useful to consider the results separately for different day types and time slices. This is because the traffic conditions are different and therefore so are the impacts. Table 1-1 shows the time slices and day types used for the flow and journey time analysis, in accordance with the Monitoring Design Report.

| Table 1-1 Day type and time slice definiti | ons |
|--|-----|
|--|-----|

| Day type | AM peak Inter-peak | | PM peak | |
|-------------------|--------------------|---------------|---------------|--|
| Monday – Thursday | 05:30 - 10:30 | 10:30 – 15:00 | 15:00 - 20:00 | |
| Friday | 05:00 - 09:00 | 09:00 - 13:00 | 13:00 – 20:00 | |
| Saturday - Sunday | | 08:00 - 20:00 | | |

1.4. Expected effects of SM-ALR

The SM-ALR concept involves increasing the number of running lanes from three to four by re-allocating the space previously used by the hard shoulder. In addition, other infrastructure is provided to deliver a controlled environment to manage the risks associated with converting the hard shoulder to a traffic lane.

The effect of an increase in capacity is that periods of congestion are expected to be less frequent, shorter and less intense leading to reductions in journey time and better journey time reliability. The road effectively becomes more resilient to regular and incident related congestion.

In addition safety benefits could be realised because traffic speeds become more consistent and the speed differential between lanes reduces. The number of unnecessary hard shoulder stops, which are inherently risky, should also be reduced.

These effects can be seen by looking at traffic performance on a daily basis. The following subsections show speed by lane, flow by lane, speed distribution and speed flow curves for typical days in the before and Yr2 after periods. The plots show a snapshot of just one location and one day, to demonstrate the positive impacts.

1.4.1. Speed by lane

The effect on speeds has been positive as demonstrated by Figure 1-4, which shows a snapshot of data from Before and Yr2 After collected during the evaluation process. It can be seen that in the Before period, there was significant congestion in AM, PM and inter-peaks. Average offside lane speeds were above the national speed limit during uncongested times (up to 78mph at night) and there was a speed differential of approximately 10mph between lanes. The After snapshot shows congestion only in the AM peak, lasting for a shorter duration. The average offside lane speeds have reduced closer to the speed limit and the speed differential between lanes is also lower, in the order of 6mph.







1.4.2. Flow by lane

Figure 1-5 shows the flow by lane for the Before and Yr2 After periods. In the Before, flows per lane are relatively similar for most of the day. The lane 3 flow is the highest in the peak periods and the lowest during the night.







It is notable that lane 1 flows are much lower than the other lanes in Yr2 After, indicating that drivers are less keen to use that lane; this effect is common when SM schemes are introduced. The lane gain, lane drop arrangement can make lane 1 less likely to be used; in addition drivers can be reluctant to use lane 1 as found in the road user surveys performed in the first year of operation.

Another useful finding demonstrated by these graphs is that although total flow is higher, the peak flow per lane is lower in Yr2 After compared to the Before, this indicates that there is still some spare capacity for future growth.

1.4.3. Speed distribution

Figure 1-6 shows the approximate proportions¹ of vehicles travelling at speeds in different 10mph 'bands', over a 24 hour period in the Before and Yr2 After. The key points of interest are:

In the Yr2 After period, over 85% of vehicles were travelling between 51 and 70mph, compared to 65% in the Before. A larger proportion of vehicles were doing low speeds in the Before period due to greater levels of congestion. In addition, a greater proportion of vehicles exceeded the speed limit in the Before, including some doing over 81mph.



Figure 1-6 Speed distribution Before and Yr 2 After

¹ TCD data has been used providing the average speed minutely per lane.

1.4.4. Speed flow curve

In the Before period it can be seen that traffic flow peaks at 115veh/min, before the flow breaks down causing congestion, while in the After Yr2 sample flows go up to 120veh/min with noticeably less severe flow breakdown.





2. Flows

2.1. Introduction

This section presents the results of the traffic flow analysis for J23 to J26; it has not been possible to evaluate the flows between J26 and J27 due to issues with the traffic data. At the Yr1 After stage, no traffic data was presented for J25 to J27. Whilst it was envisaged that the quality of the available traffic data at the Yr2 After stage would have improved sufficiently to allow a full analysis, this has only been possible for J25 to J26 due to ongoing data quality issues between J26 and J27.

The traffic data has been taken from Highways England's MIDAS database. Similar data is available through the WebTRIS website, but at present the website does not present data for the section considered here.

The Yr2 After period covers the second year after opening, May 2015 – April 2016. The Yr1 After report covered the six months following the opening of Section 5b (J25 - J27); these results are not comparable with the 12 month sample so have not been included in this report.

2.1.1. Data availability and quality

Highways England and their suppliers are investigating a known issue with the quality of flow data from radar detectors and work is underway to improve it. Until the data quality improves, to overcome the variability between adjacent detectors during the evaluation period, data from all detector sites have been compared against publically available Manual Classified Counts (MCCs) to determine which sites can be considered to be more reliable. It was not possible to do this for J26 to J27 due to the lack of MCC data in the time period being evaluated, so no comparison has been performed for this link.

2.2. Daily flows per link

The average daily traffic for the Before and After periods is compared in Figure 2-1 to Figure 2-6, with the 24 hour Average Daily Traffic (ADT) flows between each junction plotted for the different day types. The percentage change is shown above the Yr2 After bar in each case. The corresponding values are shown in Appendix A.1.

Daily flows have increased in all day types in both directions.

In the clockwise direction, weekday flows have seen increases between 8% and 9% for Mon-Thu and Fri day types between J23 and J24 and between J25 and J26. The clockwise traffic flow between J24 and J25 is 14% and 13% for the Mon-Thu and Fri day types respectively. Whilst this cannot be directly compared to the traffic flow changes at the Yr1 After stage (as only six months of data was being considered) it should be noted that there was minimal increase in traffic between J24 and J25 at that stage. Traffic flow increases at the weekend are between 15% and 16% for J23 to J24 and J25 to J26; again a greater increase of 26% can be seen between J24 and J25.

In the anticlockwise direction, the largest traffic flow increase is again seen between J24 and J25 (at 11% for Mon-Thu and Fri day types) but the traffic flow increase between J25 and J26 is very similar at 10% for the corresponding day types. The traffic flow increase between J23 and J24 is slightly lower at 6% and 8% for the Mon-Thu and Fri day types respectively. Traffic flow increases at the weekend are again higher than for weekdays, with increases of between 14% and 18% being observed.







Figure 2-2 Average daily traffic by day type J24-J25 clockwise



Figure 2-3 Average daily traffic by day type J25-J26 clockwise













The overall AADT over the 12 months of Before and Yr2 After traffic data has increased by between 8% (J23 to J24 anticlockwise) and 17% (J24 to J25 clockwise), with most other sections/directions experiencing an increase in traffic flow of between 10% and 12%.

This can be considered against South East regional motorway traffic increasing between 2012 and 2015 by around $5\%^2$. The growth along the scheme is far above that observed in the regional trends. Although the increase in traffic flows on the M25, facilitated by the scheme, will in itself contribute to the regional traffic trends, this is still a useful point to note when looking at link by link flow increases.

2.3. Flow over each time slice per link

Figure 2-7 to Figure 2-11 compare the average Before and Yr2 After flows by time slice for each link. The percentage change is shown above the Yr2 After bar in each case. The corresponding values are shown in Appendix A.2, with significant changes shown in bold text.

Clockwise, there are large flow increases on all links in all time slices. The largest increase can be seen at the weekend at 26%. The lowest increase is during the Friday inter-peak between J25 and J26 at 1%. The Mon-Thu AM peak for the same section is also lower than other time slices with an increase of 3%. In general, the largest increases in traffic flow can be seen between junctions J24 and J25.



Figure 2-7 Average flow by time slice J23-J24 clockwise

² <u>https://www.gov.uk/government/collections/road-traffic-statistics</u>









Similarly to the daily traffic flows, most links and time slices see a large increase in traffic in the anticlockwise direction. The traffic flow increase is generally lower between J23 and J24. The largest increase is again seen between J24 and J25, with similar increases seen between J25 and J26. The largest increase is over

the weekend between J24 and J25 and the smallest increases are during the Mon-Thu AM peak and the Friday PM peak between junctions J23 and J24.



Figure 2-10 Average flow by time slice J23-J24 anticlockwise







Figure 2-12 Average flow by time slice J25-J26 anticlockwise

2.4. Tests for statistical significance of the results

A sequence of t-tests was performed in order to assess whether the changes in the flows measured have been significant. In order to carry out these t-tests, the standard deviation, average flow and number of observations were calculated for each link in the section. An observation was considered to be a full day of data for each site on the link which was considered. This was required as the data has had to be collected and averaged for a number of sites in order to address the variability in the observations. The t-tests assumed that there was no change in the flow and tested for a statistically significant change using a two-tailed test at a 95% confidence level.

The results are presented in Table 2-1 for the flows by time slice and in Table 2-2 for the ADTs; the results are presented by a series of arrows denoting the type of change experienced in that time period. An up arrow denotes a statistically significant increase in flow, a down arrow denotes a statistically significant decrease, and a dash denotes no significant change in the time period.

| | | Mon-Thurs | | | | Seturday | | | |
|---------------|-----------|------------|----------------|------------|------------|----------------|------------|------------|--|
| Direction | Location | AM Peak | Inter- peak | PM Peak | AM Peak | Inter- peak | PM Peak | Sunday | |
| Clockwise | J23 - J24 | \uparrow | \uparrow | \uparrow | \uparrow | \uparrow | \uparrow | \uparrow | |
| Clockwise | J24 - J25 | \uparrow | \uparrow | \uparrow | \uparrow | \uparrow | \uparrow | \uparrow | |
| Clockwise | J25 - J26 | - | - | - | - | - | - | - | |
| Anticlockwise | J23 - J24 | - | \uparrow | \uparrow | \uparrow | \uparrow | \uparrow | \uparrow | |
| Anticlockwise | J24 - J25 | \uparrow | \uparrow | \uparrow | \uparrow | \uparrow | \uparrow | \uparrow | |
| Anticlockwise | J25 - J26 | \uparrow | \uparrow | \uparrow | \uparrow | \uparrow | \uparrow | \uparrow | |

Table 2-1Flow by time slice t-tests

Table 2-2 ADT t-tests

| Direction | Location | Mon-Thurs | Friday | Sat-Sun | ADT |
|---------------|-----------|------------|------------|------------|------------|
| Clockwise | J23 - J24 | \uparrow | \uparrow | \uparrow | \uparrow |
| Clockwise | J24 - J25 | \uparrow | \uparrow | \uparrow | \uparrow |
| Clockwise | J25 - J26 | - | - | - | - |
| Anticlockwise | J23 - J24 | \uparrow | \uparrow | \uparrow | \uparrow |
| Anticlockwise | J24 - J25 | \uparrow | \uparrow | \uparrow | \uparrow |
| Anticlockwise | J25 - J26 | \uparrow | \uparrow | \uparrow | \uparrow |

From these results, it can be seen that the majority of the traffic flow changes have been shown to be significant. It should, however, be noted that no accommodation has been made for the background change in traffic levels which will in part have contributed to the growth along the scheme. No analysis has been completed between J25 and J26 clockwise as a result of the traffic data availability issues on this section.

2.5. Summary

The SM-ALR section has experienced traffic growth of over 10% between the Before and Yr2 After periods, which is far higher than regional motorway growth over the same period. The largest growth has generally been in the peak periods and at weekends, although anticlockwise J24 to J25 and J25 to J26 inter-peaks have also seen a high level of traffic growth.

3. Journey times

3.1. Introduction

This section outlines the changes in journey times and reliability on the M25 J23-27 SM-ALR between the Before and Yr2 After periods.

The data used was supplied by TomTom who provide anonymised data of journeys through the scheme during the Before and After periods. The journey time data is at a very spatially disaggregate level, allowing speed analysis to be undertaken at regular intervals along the scheme.

Before interrogating the TomTom database, a review of severe incidents and road works was undertaken to identify any days that should be removed from the analysis because they would not represent normal operating conditions. No such days were identified in the samples, therefore all days within the year are included in the dataset.

The journey time results presented in this section form the latest conclusions on journey time performance for the scheme following two years of operation.

3.2. Average journey time

The analysis of average journey times from junction to junction demonstrates the change in journey times at link level. The headline results are summarised in Figure 3-1 for clockwise and Figure 3-2 for anticlockwise with more detail provided in Appendix B.

Clockwise, between the Before and Yr2 After periods, there has been an average percentage increase of 3% (i.e. 26 seconds) across all day types and time slices from the average Before journey time of 15 minutes 47 seconds. The most congested periods were the weekday PM peaks and they have experienced worsened journey times (of 8% Mon-Thu and 5% Fri).



Figure 3-1 Clockwise journey time comparison

SM-ALR Monitoring M25 J23-27 Second Year Evaluation Report

On individual links, the scheme has provided a slight journey time benefit on the J23-24 link with an overall decrease in average journey time of 7 seconds across all day types and time slices. The J24-25, J25-26 and J26-27 links have seen average journey time increases of 23 seconds, 2 seconds and 7 seconds respectively. So it appears that the J24-25 link has caused most of the additional delay in the After period. A review of congestion using MTV plots have revealed that there is often significant congestion approaching J25; sometimes the cause appears to be in the vicinity of the off-slip and sometimes in the region of the onslip and Holmesdale Tunnel.

The roundabout at J25 has been the subject of review by Highways England in conjunction with TfL and other local authorities. It is currently being upgraded with new traffic signals equipment which could reduce the congestion in this area in the near future.



Figure 3-2 Anticlockwise journey time comparison

Anticlockwise, between the Before and Yr2 After periods, there has been an average percentage reduction of 0.5% (i.e. 5 seconds) across all day types and time slices. This means effectively no change in journey times from the average Before journey time of 16 minutes 37 seconds. The Mon-Thu AM peak periods experience the worst congestion; there has been a 5% journey time improvement but this is balanced by a worsening in the IP period.

The scheme has provided journey time benefits on the J24-25 link of 5 seconds across all day types and time slices. On the other links the journey times were virtually unchanged across all day types and time slices, although there were differences between the periods.

In summary, average journey times are nearly half a minute longer in the clockwise direction and almost unchanged anticlockwise.

3.3. Journey time reliability

Reliability of journey times is a critical measure of a road's utility and function for road users. Percentile data has been used to understand the distribution of journey times through the scheme. Four metrics have been used, as shown in Table 3-1.

| Metric | Description |
|-----------------------------|--|
| 5 th percentile | One in 20 vehicles are completing the journey faster than this, so it is a good measure of the best time achievable. |
| 25 th percentile | One in four vehicles are completing the journey faster than this and it is known as the lower quartile. The further this value is from the 5th percentile the more variability there is in the fastest journeys. It is an indicator that delays are experienced by a high proportion of all users. |
| 75 th percentile | Three quarters of vehicles complete the journey faster than this and it is a good measure of general variability from day to day of journey times. |
| 95 th percentile | 95% of vehicles complete the journey faster than this, the remaining journeys are likely to be affected by incidents or heavy congestion. The further the 95th percentile journey time is from the 75 th percentile the more heavily congested a journey is. This is an indication of incident related variability. |

Table 3-1 Journey time metrics

These four metrics are shown below in Figure 3-3 and Figure 3-4 as 'box and whisker' diagrams for each time slice, Before and Yr2 After. The box contains the 25th to 75th percentile range and the whiskers show the 5th and 95th percentile values. The 75th percentile and 95th percentile journey times are annotated on the plots.



Figure 3-3 Clockwise journey time reliability analysis

Clockwise, the most unreliable journey times, Before and Yr2 After, are during the weekday PM peaks. Although the 95th percentile values of the weekday PM peaks increased, the variability in journey time experienced by the majority of road users during these periods has decreased (i.e. a reduced difference

between the 25th and 75th percentile, the interquartile range). So journeys in the PM peaks are longer but more reliable; possibly due to the action of the variable mandatory speed limits.

In all other weekday periods, the interquartile range has increased very slightly, in the range of four to 21 seconds. This might be because when a vehicle stops in a live lane it is likely to cause more disruption than previously when the hard shoulder was available.



Figure 3-4 Anticlockwise journey time reliability analysis

Anticlockwise, the most unreliable journey time, Before and Yr2 After, is in the Monday-Thursday AM peak. This has experienced reductions in both the 95th percentile and the 75th percentile as well as the interquartile range. This shows better journey time reliability with the all lane running scheme.

In other (previously uncongested) periods, the interquartile range is similar between the Before and Yr1 After period, showing no change in day-to-day reliability.

3.4. Summary

Overall clockwise journey times have increased by 26 seconds (3%) in Yr2 After compared to Before. In the anticlockwise direction journey times have remained similar.

In both directions, delays have been identified with the cause in the region of J25. The roundabout at J25 has been the subject of review by Highways England in conjunction with TfL and other local authorities. It is currently being upgraded with new traffic signals equipment which could reduce the congestion in this area in the near future.

Journey time reliability clockwise remains similar between the Before and Yr2 After period using the interquartile measure, however the most delayed journeys as measured by the 95th percentile have noticeably increased in weekday PM peaks. Anticlockwise there is a good improvement on the Monday to Thursday AM time slice, while other time slices remain similar.

These results show that increases in capacity have been achieved, moving more goods, people and services, while maintaining journey times at pre-scheme levels and slightly improving reliability.

4. Safety

4.1. Introduction

This section compares the Before and After safety performance of the M25 J23 to J27 SM-ALR scheme. STATS19 data has been used to identify the number and rate of personal injury collisions, however it should be noted that the After period (two years for J23 to J25 and 18 months for J25 to J27) is a relatively short sample period; a larger data set is required before the findings will become statistically significant and conclusions can be drawn. (The minimum desirable period for collision data analysis results to become statistically significant is three years.) However emerging safety data provides an extremely beneficial insight into the performance of ALR.

STATS19 collates all injury collision data in a consistent manner each year and is a generally reliable source for numbers of injury collisions. Damage-only collisions are not recorded in STATS19 so it is not a record of all collisions. Recording collision details relies on police input at the collision scene, therefore there is some scope for inconsistencies when the information is recorded. This data is robust to the extent that it is unlikely to change significantly when the validated results are produced.

This section also contains analysis of Red X and speed limit compliance and ERA usage in the After period.

Note that it was not possible to measure flows accurately between J25 and J27, so the flows used for the rate calculations have been estimated by comparing with adjacent links and alternative data sources (DfT manual count data). This means the rate calculations must be treated with caution.

4.2. Number and rate of collisions

Table 4-1 shows the number of collisions during the Before and After periods, the rate of collisions and the percentage change. The number of collisions is exactly half in the After period which is half as long as the Before, however the traffic flows have increased leading to a slight reduction in collision rate. To fully understand the results we also need to take into account the background trend in collisions, see Section 4.2.1.

| Table 4-1 | Number of | collisions | by severity | and collision | rates |
|-----------|-----------|------------|-------------|---------------|-------|
|-----------|-----------|------------|-------------|---------------|-------|

| Period | | Fatal | Serious | Fatal & serious | Slight | Total |
|--------|---|-------|---------|-----------------|--------|--------|
| | Year 1 | 1 | 11 | 12 | 85 | 97 |
| | Year 2 | 2 | 11 | 13 | 88 | 101 |
| Roforo | Year 3 | 1 | 5 | 6 | 87 | 93 |
| Delore | Total | 4 | 27 | 31 | 260 | 291 |
| | Collision rate (collisions per hmvm) (22.6 hmvm) | 0.177 | 1.194 | 1.371 | 11.500 | 12.871 |
| | Collision rate (collisions per mvkm) (3,641 mvkm) | 0.001 | 0.007 | 0.009 | 0.071 | 0.080 |
| | Year 1 | 2 | 3 | 5 | 55 | 60 |
| After* | Year 2 | 1 | 9 | 10 | 93 | 103 |
| | Total | 3 | 12 | 15 | 148 | 163 |
| | Collision rate (collisions per hmvm) (14.2 hmvm) | 0.212 | 0.847 | 1.059 | 10.444 | 11.502 |
| | Collision rate (collisions per mvkm) (2,281 mvkm) | 0.001 | 0.005 | 0.007 | 0.065 | 0.071 |

*Due to staged opening this is based on 24 months of data for J23 to J25 and 18 months of data for J25 to J27. Total values cannot be compared between Before and After periods, but rates can be compared.

The four fatal collisions in the Before period include:

- A car transporter losing control for unknown reasons and striking street lighting;
- A motorcyclist weaving through traffic and colliding with a vehicle;
- A vehicle drifting across lanes and losing control; and
- A suspected suicide where a pedestrian ran out into the carriageway.

There were three fatal collisions in the After period, as follows:

- A stowaway incident where a pedestrian climbed out from underneath a vehicle and was run over;
- A suspected suicide attempt where a car pulled out from the nearside verge (actually a very short length
 of hard shoulder) into the path of an HGV in lane 1 causing the HGV to swerve and collide with another
 HGV causing a crossover and ultimately the fatality of an HGV occupant on the opposite carriageway;
 and
- A slow moving / stationary vehicle in lane 1 was struck by another vehicle.

There were a total of 12 serious collisions in the After period, as follows:

- Four nose to tail collisions:
 - A bus or coach braked and was hit from behind by an HGV;
 - An HGV collided with a van in front for unknown reasons;
 - Two cars were braking and the second vehicle was hit from behind by a third car which pushed the second vehicle into the first vehicle; and
 - A car was hit from behind by an HGV which pushed it into a car in front. The front vehicle lost control, striking the central reserve barrier and rebounded across the carriageway into the nearside barrier and off the motorway. A toddler in the middle vehicle was being carried on the lap of a pregnant passenger and suffered serious injuries.
- Three collisions associated with lane changing and/or failing to look:
 - An HGV moved from lane 1 into lane 2, colliding with a van in lane 2;
 - A car moved from lane 3 into lane 2, colliding with a car in lane 2. The first car left the scene; and
 - A car moved from lane 1 of the slip road for the M11 Northbound into lane 2 for the M11 Southbound. A motorbike already in lane 2 could not stop and hit the rear of the car, unseating the rider.
- Four single vehicle collisions:
 - A driver who had fallen asleep woke as their car was about to collide with an HGV, panicked and lost control, striking the nearside and then offside tunnel walls;
 - A vehicle left the motorway to the offside for unknown reasons, struck the central reserve barrier and caught fire;
 - A vehicle left the motorway to the nearside, struck the tunnel wall and came to rest in the hard shoulder; and
 - An HGV lost control and turned onto its side.
- One other collision where a car entered a slip road which was closed for recovery work and collided with a road worker. The car failed to stop at the scene.

It can be seen the serious collisions could have occurred on any stretch of motorway so are not attributable to ALR. The contributory factors by severity for the collisions are shown Appendix C.1.

4.2.1. Background trend in collisions

There is a trend over time leading to a reduction in the number of personal injury collisions against a trend of increasing traffic volumes. The reasons for the reduction are wide ranging and include improved safety measures in vehicles. This trend needs to be accounted for when comparing the Before and After periods.

The best way to take into account the national trend is to assume that, if the scheme had not been built, the number of collisions on the roads in the study area would have dropped at the same rate as they did nationally during the same time period. This provides what is known as a counterfactual 'without scheme' scenario and can be compared on a like-for-like basis with the observed After data which is the 'with

scheme' scenario³. The difference between the numbers of collisions in these two scenarios can then be attributed to the scheme rather than the wider national trends.

Table 4-2 shows that there has been a very small increase in the collision rate of 1% over and above the background reduction in collisions (compared to the 11% absolute reduction in Table 4-1).

| Period | Number of collisions | Collision rate (collisions per hmvm) | Collision rate (collisions per mvkm) | |
|-------------------------------|----------------------|---|---|--|
| Annual average Before period | 97.00 | 12.87 | 0.080 | |
| Counter factual Before period | 92.52 | 11.41 | 0.071 | |
| After* | 163 | 11 50 | 0.071 | |
| Annual average After period | 93.14 | 11.50 | 0.071 | |

Table 4-2 Number of collisions and collision rates following national trends

*Due to staged opening this is based on 24 months of data for J23 to J25 and 18 months of data for J25 to J27. Total values cannot be compared between Before and After periods, but rates can be compared.

4.2.2. Statistical significance

A Chi squared test compared the number of Before and After collisions and Annual Average Daily Traffic flows (AADTs) against expected values if there was no change. The test result indicates that the change in the collision rate is not statistically significant and therefore not necessarily a direct impact of the scheme. This means that neither the 11% reduction, nor the 1% increase compared to background trends, can be attributed to the scheme.

4.3. Casualties, FWI and KSI rate

Fatal Weighted Injury (FWI)⁴ is calculated based on the number of fatal, serious and slight casualties as weighted proportions, to adjust for the severity. The FWI rate allows a comparison between road sections of different flows and lengths.

The slight reduction in the FWI rate shown in Table 4-3 is attributable to the lower number of serious and slight casualties in the After period, however this is based on a small After sample size so is not statistically significant.

Table 4-3 Number of casualties and FWI rate

| Pariod | | Severity | | Total | E\0/I | FWI rate | FWI rate |
|--|-------|----------|--------|-------|-------|----------|----------|
| renou | Fatal | Serious | Slight | TOTAL | L AAI | hmvm | bvkm |
| Before (36 months) (22.6 hmvm, 3.64 bvkm) | 4 | 36 | 401 | 441 | 11.61 | 0.51 | 3.19 |
| After* (14.2 hmvm, 2.28 bvkm) | 3 | 16 | 233 | 252 | 6.93 | 0.49 | 3.04 |

*Due to staged opening this is based on 24 months of data for J23 to J25 and 18 months of data for J25 to J27. Total values cannot be compared between Before and After periods, but rates can be compared.

There has been a reduction in the Killed or Seriously Injured (KSI) rate, shown in Table 4-4, which is attributable to the lower number of serious casualties in the After period. However this is based on a small After sample size so, again, is not statistically significant.

³ The counterfactual factor is calculated using the national collision data for motorway class roads in the After period (2015) and for the middle year in the Before period (2011). The calculated factor between these years is 0.95 for the number of collisions and 0.89 for the collision rate.

 $^{^{4}}$ FWI is defined as: (number of fatalities) + 0.1 x (number of serious casualties) + 0.01 x (number of slight casualties).

Table 4-4Total KSI and KSI rate

| Period | Total KSI | KSI rate per hmvm | KSI rate per bvkm |
|--|-----------|-------------------|-------------------|
| Before (36 months) (22.6 hmvm, 3.64 bvkm) | 40 | 1.77 | 10.99 |
| After* (14.2 hmvm, 2.28 bvkm) | 19 | 1.34 | 8.33 |

*Due to staged opening this is based on 24 months of data for J23 to J25 and 18 months of data for J25 to J27. Total values cannot be compared between Before and After periods, but rates can be compared.

4.4. Additional analysis

4.4.1. Red X (lane closed) analysis

An analysis of Red X compliance was undertaken using HALOGEN data for Sign and Signal settings and MIDAS TCD files for minutely flows per lane. The two data sets were combined to identify lane closures and flows along the lane during the restriction. An example of a Red X event is presented in Figure 4-1.

| | | Detector/ | Signal | | Minutely | Flow Dur | ing Lane | Closure |
|------------------------|------------|-----------|---------|---|----------|----------|----------|---------|
| | | Locati | ion | MS4 Message | L1 | L2 | L3 | L4 |
| | | M2 | 5/5413B | | 7 | 21 | 30 | 32 |
| | MS4 | 5413B | | NR | | | | |
| | | M23 | 5/5419B | | 0 | 12 | 11 | 10 |
| | MS4 & AMIs | 5419B | | LANE CLOSED FOR INCIDENT ACCESS | *X* | (40) | (40) | (40) |
| | | M2: | 5/5424B | | 1 | 24 | 29 | 32 |
| | | M2 | 5/5429B | | 2 | 21 | 30 | 28 |
| | | M2 | 5/5432B | | 1 | 22 | 28 | 29 |
| Direction of travel | MS4 | 5432B | | DO NOT USE CLOSED LANES | | | | |
| | | M25 | 5/5437B | | 2 | 21 | 25 | 28 |
| Î | | M25 | 5/5441B | | 1 | 20 | 27 | 27 |
| | | M25 | 5/5445B | | 2 | 23 | 24 | 28 |
| | MS4 | 5445B | | DO NOT USE CLOSED LANES XIII (40) | | | | |
| | | M2 | 5/5449B | | 2 | 21 | 25 | 26 |
| | | M2 | 5/5453B | | 1 | 21 | 28 | 28 |
| | MS4 | 5453B | | Congestion stay in lane /III (50) | | | | |
| | | M25 | 5/5458B | | 3 | 21 | 24 | 28 |
| | J25 Merge | M25 | 5/5464B | | 2 | 14 | 15 | 15 |

Figure 4-1 Example lane closure event

Key:

| 5381B | | Signal |
|-------|-----------|---|
| | M25/5381B | Detector site |
| L1 | | Lane 1 |
| L2 | | Lane 2 |
| L3 | | Lane 3 |
| L4 | | Lane 4 |
| | 60 | 60mph VMSL |
| | 50 | 50mph VMSL |
| | 40 | 40mph VMSL |
| l | LDR | Lane Divert Right Arrow |
| | | Red X on gantry indicating closed lane |
| | ×III | Wickets on MS4 Sign showing lane 1 closure |
| | xxII | Wickets on MS4 Sign showing lane 1 and lane 2 closure |
| | /111 | Wickets on MS4 Sign showing lane divert right |
| | NR | National Speed Limit |

A total of 20 lane closures have been assessed in the Yr2 After period and the results are summarised in Appendix C.2. The per-lane minutely flow is provided to give an indication of how busy the motorway was; a flow of 30 vehicles per minute per lane is a high flow (one vehicle every 2 seconds).

Non-compliance in this sample ranges from 1 to 5 vehicles per minute, 1% to 10% of total flow; across all Red X events analysed the minutely average flow of non-compliant vehicles was 2 per minute. Compliance with Red X as a percentage of total flow was 96%, which appears to show a slight improvement from the 93% in the Yr1 After period, although the sample size is relatively low.

The percentage non-compliance was compared to the incident duration and traffic flow; no correlation was found with either. This suggests that the subset of drivers who choose not to comply with Red Xs do so regardless of how busy the motorway is or how long the incident duration is.

4.4.2. ERA monitoring

The ERA monitoring was undertaken to identify the causes of ERA stops, vehicle types and risks of entering, stopping or exiting the ERA. Six ERAs were identified for continuous monitoring covering both peaks and inter-peak. A total of 240 hours ERA monitoring was divided between the following ERAs:

- ERA 1 M25 anticlockwise between J23 and J24 at 5400B;
- ERA 2 M25 clockwise between J25 and J26 at 5501A;
- ERA 3 M25 anticlockwise between J25 and J26 at 5508B;
- ERA 4 M25 clockwise between J25 and J26 at 5511A;
- ERA 5 M25 clockwise between J25 and J26 at 5524A; and
- ERA 6 M25 anticlockwise between J26 and J27 at 5545B.

In total 119 unique ERA stops were observed. This means stops by a lead vehicle; further related vehicle activity such as Highways England Traffic Officer services or recovery vehicles are not counted. A summary of ERA activity can be seen in Table 4-5.

Table 4-5 Summary of ERA activity

| Activity | Number | Percentage of all stops |
|---|--------|----------------------------|
| Emergency Refuge Telephone (ERT) used | 4 | 3% |
| Highways England Traffic Officer attended | 8 | 7% |
| Non-emergency (e.g. drove off without exiting vehicle, comfort break etc.) | 97 | 82% |
| Genuine reason (e.g. problem with vehicle) | 22 | 18% |

The 119 unique ERA stops over 240 hours of ERAs monitored gives a rate of approximately 0.5 stops per hour per ERA. From the sample observed it was judged that 82% were non-emergency, a slight decrease from the 85% non-emergency stops in Yr1 After.

A breakdown of the types of lead vehicles which stopped in ERAs and whether they were genuine emergencies is shown in Table 4-6. As cars make up the majority of vehicles on the road it is not surprising that they account for the most stops. HGVs represented the highest non-emergency use of ERAs, at 91%. This compares with 96% of HGVs stopping for a non-emergency reason in Yr1 After, these numbers are fairly consistent. For the entire After period, the non-emergency stops for HGVs is 93%.

Other ERA observations were:

- no instances of problems with ERA operation were observed; and
- no collisions relating to vehicles exiting ERAs.

| Vehicle type | Number of ERA stops | Percentage of total | Non-emergency | Genuine emergency |
|--------------|------------------------|---------------------|---------------|----------------------|
| Car | 52 | 44% | 75% | 25% |
| Van | 30 | 25% | 83% | 17% |
| HGV | 32 | 27% | 91% | 9% |
| LGV | 5 | 4% | 80% | 20% |
| Total | 119 | | 82% | 18% |

Table 4-6 Vehicle types using ERAs

4.5. Summary

The After period STATS19 sample size is small so the results are not conclusive, although they provide a beneficial insight into the performance of ALR. There is a very small increase in the collision rate, over and above the national background trend of improved safety; this is not statistically significant and should be considered as 'no significant change'. This suggests that the scheme is achieving its objective of maintaining safety performance, although further monitoring is required due to the small sample size.

Monitoring of Red X compliance revealed that across all events analysed, an average of 4% of vehicles did not comply with Red Xs in the Yr2 After period.

Approximately 0.5 ERA stops per hour per ERA were observed during monitoring of CCTV, the same as Yr1 After monitoring. It was judged that 82% were non-emergency. HGVs make a higher proportion of ERA stops compared to the general vehicle make up and also had a very high non-emergency use of ERAs at 91%.

5. Conclusions

5.1. Flow

The SM-ALR section has experienced traffic growth of over 10% between the Before and Yr2 After periods, which is far higher than regional motorway growth over the same period. The largest growth has generally been in the peak periods and at weekends, although anticlockwise J24 to J25 and J25 to J26 inter-peaks have also seen a high level of traffic growth. No data was available for J26 to J27.

5.2. Journey time

Overall clockwise journey times have increased by 26 seconds (3%) in Yr2 After compared to Before. In the anticlockwise direction journey times have remained similar.

In both directions, delays have been identified with the cause in the region of J25. The roundabout at J25 has been the subject of review by Highways England in conjunction with TfL and other local authorities. It is currently being upgraded with new traffic signals equipment which could reduce the congestion in this area in the near future.

Journey time reliability clockwise remains similar between the Before and Yr2 After period using the interquartile measure, however the most delayed journeys as measured by the 95th percentile have noticeably increased in weekday PM peaks. Anticlockwise there is a good improvement on the Monday to Thursday AM time slice, while other time slices remain similar.

5.3. Safety

The After period STATS19 sample size is small so the results are not conclusive. There is a very small (but not statistically significant) increase in the collision rate, over and above the national background trend of improved safety. However, reductions have been seen in the severity index and the FWI and KSI rates but no conclusions should be drawn from this due to the small sample size.

Monitoring of Red X compliance revealed that across all events analysed, an average of 4% of vehicles did not comply with Red Xs in the Yr2 After period.

Approximately 0.5 ERA stops per hour per ERA were observed during monitoring of CCTV, the same as Yr1 After monitoring. It was judged that 82% were non-emergency. HGVs make a higher proportion of ERA stops compared to the general vehicle make up and also had a very high non-emergency use of ERAs at 91%.

Appendices

Appendix A. Flows additional information

A.1. 24 hour average daily traffic (ADT)

The table below shows the values for ADTs Before and After.

| | | | Cloc | kwise | | Anticlockwise | | | |
|-----------|------------------------------------|---------------|--------|-------------|--------|---------------|--------|-------------|--------|
| Location | Value | Mon- Thurs | Friday | Sat- Sun | ADT | Mon- Thurs | Friday | Sat- Sun | ADT |
| | Before | 73,700 | 77,300 | 60,500 | 70,500 | 72,600 | 76,500 | 60,000 | 69,600 |
| | Yr2 After | 79,800 | 83,600 | 70,300 | 77,600 | 77,000 | 82,300 | 69,100 | 75,500 |
| 100 104 | Change | 6,100 | 6,300 | 9,800 | 7,100 | 4,400 | 5,800 | 9,100 | 5,900 |
| JZ3 - JZ4 | % Change (against Before) | 8% | 8% | 16% | 10% | 6% | 8% | 15% | 8% |
| | Before | 69,800 | 73,500 | 55,700 | 66,300 | 67,300 | 71,100 | 55,400 | 64,400 |
| | Yr2 After | 79,500 | 83,400 | 70,200 | 77,400 | 74,400 | 78,700 | 65,500 | 72,400 |
| 10.4 105 | Change | 9,700 | 9,900 | 14,500 | 11,100 | 7,100 | 7,600 | 10,100 | 8,000 |
| J24 - J25 | % Change (against Before) | 14% | 13% | 26% | 17% | 11% | 11% | 18% | 12% |
| | Before | 64,600 | 69,200 | 54,100 | 62,300 | 65,700 | 70,400 | 55,900 | 63,600 |
| | Yr2 After | 70,100 | 74,600 | 62,200 | 68,500 | 72,300 | 77,100 | 63,600 | 70,500 |
| 105 100 | Change | 5,500 | 5,400 | 8,100 | 6,200 | 6,600 | 6,700 | 7,700 | 6,900 |
| J25 - J26 | % Change (against Before) | 9% | 8% | 15% | 10% | 10% | 10% | 14% | 11% |

A.2. Flows by time slice

| | Clockwise flows by time slice | | | | | | | | | |
|-----------|-------------------------------|------------|----------------|------------|------------|----------------|------------|-----------------|--|--|
| | | | Mon-Thur | s | | Friday | | | | |
| Location | Value | AM Peak | Inter- peak | PM Peak | AM Peak | Inter- peak | PM Peak | Saturday-Sunday | | |
| | Before | 20,700 | 18,700 | 24,200 | 14,100 | 16,400 | 34,700 | 46,000 | | |
| | Yr2 After | 21,700 | 20,000 | 26,100 | 15,500 | 17,400 | 36,900 | 53,000 | | |
| J23 - J24 | Change | 1,000 | 1,300 | 1,900 | 1,400 | 1,000 | 2,200 | 7,000 | | |
| | % Change | 5% | 7% | 8% | 10% | 6% | 6% | 15% | | |
| | Before | 19,000 | 17,700 | 23,300 | 13,000 | 15,500 | 33,500 | 42,000 | | |
| | Yr2 After | 21,500 | 19,900 | 26,100 | 15,400 | 17,300 | 36,700 | 52,900 | | |
| J24 - J25 | Change | 2,500 | 2,200 | 2,800 | 2,400 | 1,800 | 3,200 | 10,900 | | |
| | % Change | 13% | 12% | 12% | 18% | 12% | 10% | 26% | | |
| | Before | 17,200 | 16,500 | 21,900 | 11,800 | 14,600 | 32,000 | 41,200 | | |
| J25 - J26 | Yr2 After | 17,800 | 17,600 | 23,900 | 12,800 | 14,700 | 34,200 | 46,800 | | |
| | Change | 600 | 1,100 | 2,000 | 1,000 | 100 | 2,200 | 5,600 | | |
| | % Change | 3% | 7% | 9% | 8% | 1% | 7% | 14% | | |

The table below shows the flows for each time slice in the clockwise direction.

The table below shows the flows for each time slice in the anticlockwise direction.

| | Anticlockwise flows by time slice | | | | | | | | | |
|-----------|-----------------------------------|------------|----------------|------------|------------|----------------|------------|-----------------|--|--|
| | | Γ | Mon-Thur៖ | 6 | | Friday | | | | |
| Location | Value | AM Peak | Inter- peak | PM Peak | AM Peak | Inter- peak | PM Peak | Saturday-Sunday | | |
| | Before | 23,200 | 18,400 | 21,300 | 16,100 | 18,200 | 31,900 | 45,700 | | |
| 102 104 | Yr2 After | 24,000 | 19,500 | 22,300 | 17,900 | 19,400 | 33,300 | 52,400 | | |
| JZ3 - JZ4 | Change | 800 | 1,100 | 1,000 | 1,800 | 1,200 | 1,400 | 6,700 | | |
| | % Change | 3% | 6% | 5% | 11% | 7% | 4% | 15% | | |
| | Before | 21,800 | 17,000 | 19,300 | 15,700 | 17,000 | 29,100 | 41,900 | | |
| 124 125 | Yr2 After | 24,600 | 18,900 | 20,300 | 18,400 | 18,900 | 30,500 | 49,500 | | |
| JZ4 - JZ5 | Change | 2,800 | 1,900 | 1,000 | 2,700 | 1,900 | 1,400 | 7,600 | | |
| | % Change | 13% | 11% | 5% | 17% | 11% | 5% | 18% | | |
| | Before | 21,400 | 16,200 | 19,000 | 15,400 | 16,400 | 29,000 | 42,300 | | |
| J25 - J26 | Yr2 After | 24,100 | 17,800 | 20,500 | 17,800 | 17,900 | 31,000 | 48,300 | | |
| | Change | 2,700 | 1,600 | 1,500 | 2,400 | 1,500 | 2,000 | 6,000 | | |
| | % Change | 13% | 10% | 8% | 16% | 9% | 7% | 14% | | |

Appendix B. Journey times additional information

Days in sample B.1.

The table below shows the date ranges and number of days used in the data set for the analysis:

| Period | | Clockwise | Anticlockwise | | |
|-----------|----------------|-----------|---------------|--|--|
| | From | 1 Feb 12 | 1 Feb 12 | | |
| Poforo | То | 31 Jan 13 | 31 Jan 13 | | |
| Deloie | Days removed | 0 | 0 | | |
| | Days in sample | 365 | 365 | | |
| | From | 1 May 15 | 1 May 15 | | |
| Yr2 After | То | 30 Apr 16 | 30 Apr 16 | | |
| | Days removed | 0 | 0 | | |
| | Days in sample | 365 | 365 | | |

Journey time B.2.

B.2.1. Average journey time

The tables below show the results. Where Yr2 After period journey times have become longer they are highlighted in red.

| ngniight | ea in rea. | | | | | | | | | |
|-----------------------------------|------------|---------------------|-----------|--------|--------|-------|-------|-------|-------|-------------------------------|
| Clockwise Journey Time Comparison | | | | | | | | | | |
| | Section | Distance (miles) | M-T AM | M-T IP | М-Т РМ | FAM | F IP | FPM | SS | Period average % change |
| | J23 to J24 | 2.5 | 02:37 | 02:27 | 03:15 | 02:31 | 02:24 | 03:27 | 02:22 | |
| Before | J24 to J25 | 5.6 | 05:19 | 05:14 | 05:44 | 05:18 | 05:10 | 05:59 | 04:59 | |
| | J25 to J26 | 3.6 | 03:28 | 03:29 | 03:47 | 03:29 | 03:30 | 04:01 | 03:20 | |
| | J26 to J27 | 4.4 | 04:03 | 04:06 | 04:11 | 04:07 | 04:08 | 04:13 | 03:52 | |
| | Total | 16.1 | 15:26 | 15:17 | 16:57 | 15:24 | 15:13 | 17:40 | 14:32 | |
| | J23 to J24 | 2.5 | 02:30 | 02:29 | 02:58 | 02:26 | 02:24 | 03:09 | 02:20 | -4% |
| | J24 to J25 | 5.6 | 05:28 | 05:28 | 06:48 | 05:20 | 05:17 | 06:59 | 05:06 | 7% |
| Yr2 After | J25 to J26 | 3.6 | 03:30 | 03:33 | 03:56 | 03:29 | 03:30 | 03:58 | 03:21 | 1% |
| / (10) | J26 to J27 | 4.4 | 04:06 | 04:13 | 04:32 | 04:09 | 04:08 | 04:31 | 03:53 | 3% |
| | Total | 16.1 | 15:33 | 15:42 | 18:14 | 15:24 | 15:19 | 18:36 | 14:40 | 3% |
| | % Change | | 0.8% | 2.8% | 7.5% | 0.0% | 0.7% | 5.3% | 0.9% | |
| | | | | | | | | | | |

С

Anticlockwise journey time comparison

| | Section | Distance (miles) | M-T AM | M-T IP | М-Т РМ | FAM | F IP | F PM | SS | Period average % change |
|--------|------------|---------------------|-----------|--------|--------|-------|-------|-------|-------|-------------------------------|
| | J27 to J26 | 2.5 | 05:37 | 04:03 | 04:09 | 04:21 | 04:12 | 04:06 | 03:54 | |
| Defere | J26 to J25 | 5.6 | 05:13 | 03:27 | 03:30 | 03:52 | 03:49 | 03:32 | 03:23 | |
| Before | J25 to J24 | 3.6 | 06:47 | 05:25 | 05:39 | 06:07 | 05:47 | 05:40 | 05:20 | |
| | J24 to J23 | 4.4 | 02:48 | 02:31 | 02:37 | 02:42 | 02:38 | 02:36 | 02:28 | |
| | Total | 16.1 | 20:25 | 15:27 | 15:55 | 17:02 | 16:27 | 15:55 | 15:05 | |
| | J27 to J26 | 2.5 | 05:00 | 04:15 | 04:10 | 04:21 | 04:15 | 04:09 | 03:56 | -1% |
| Yr2 | J26 to J25 | 5.6 | 04:45 | 03:40 | 03:39 | 03:49 | 03:45 | 03:43 | 03:22 | 0% |
| After | J25 to J24 | 3.6 | 06:24 | 05:39 | 05:40 | 05:53 | 05:36 | 05:40 | 05:18 | -1% |
| | J24 to J23 | 4.4 | 03:14 | 02:30 | 02:32 | 02:55 | 02:34 | 02:35 | 02:21 | 2% |
| | Total | 16.1 | 19:24 | 16:06 | 16:00 | 16:58 | 16:11 | 16:08 | 14:57 | -0.5% |
| | % Change | | -5.0% | 4.2% | 0.5% | -0.5% | -1.6% | 1.3% | -0.8% | |

Appendix C. Safety additional information

C.1. Contributory factors

Contributory factors by severity before period

| Code | Contributory factor group | Fatal | Serious | Slight | Total |
|---------|------------------------------|-------|---------|--------|-------|
| 101-109 | Road environment contributed | 0 | 2 | 21 | 23 |
| 201-206 | Vehicle defects | 2 | 3 | 6 | 11 |
| 301-310 | Injudicious action | 0 | 7 | 123 | 130 |
| 401-410 | Driver/rider error | 8 | 41 | 374 | 423 |
| 501-510 | Impairment or distraction | 0 | 7 | 37 | 44 |
| 601-607 | Behaviour or inexperience | 1 | 5 | 67 | 73 |
| 701-710 | Vision affected | 0 | 3 | 35 | 38 |
| 801-810 | Pedestrian involved | 2 | 0 | 1 | 3 |
| 901-999 | Special codes | 0 | 1 | 8 | 9 |
| Total | | 13 | 69 | 672 | 754 |

M25 J23-27 Before Period - Total Collisions by Contributory Factor Group



- Road Environment Contributed
- Vehicle Defects
- Injudicious Action
- Driver/Rider Error
- Impairment or Distraction
- Behaviour or Inexperience
- Vision Affected
- Pedestrian Involved
- Special Codes

| Rank | 1 to 10 |
|------|---------|
| Rank | 11 to |
| | 15 |

| Code | Contributory factors | Fatal | Serious | Slight | Total | Rank |
|---------|---|-------|---------|--------|-------|---------|
| Road e | environment contributed | 0 | 2 | 21 | 23 | |
| 101 | Poor or defective road surface | 0 | 0 | 0 | 0 | |
| 102 | Deposit on road (e.g. oil, mud, chippings) | 0 | 0 | 1 | 1 | 34 |
| 103 | Slippery road (due to weather) | 0 | 2 | 15 | 17 | 11 |
| 104 | Inadequate or masked signs or road markings | 0 | 0 | 0 | 0 | |
| 105 | Defective traffic signals | 0 | 0 | 0 | 0 | |
| 106 | Traffic calming (e.g. speed cushions, road humps, chicanes) | 0 | 0 | 0 | 0 | |
| 107 | Temporary road layout (e.g. contraflow) | 0 | 0 | 0 | 0 | |
| 108 | Road layout (e.g. bend, hill, narrow carriageway) | 0 | 0 | 3 | 3 | 24 |
| 109 | Animal or object in carriageway | 0 | 0 | 2 | 2 | 28 |
| Vehicl | e defects | 2 | 3 | 6 | 11 | |
| 201 | Tyres illegal, defective or under-inflated | 0 | 1 | 2 | 3 | 24 |
| 202 | Defective lights or indicators | 0 | 0 | 0 | 0 | |
| 203 | Defective brakes | 1 | 0 | 1 | 2 | 28 |
| 204 | Defective steering or suspension | 1 | 0 | 1 | 2 | 28 |
| 205 | Defective or missing mirrors | 0 | 0 | 0 | 0 | |
| 206 | Overloaded or poorly loaded vehicle or trailer | 0 | 2 | 2 | 4 | 21 |
| Injudio | ious action | 0 | 7 | 123 | 130 | |
| 301 | Disobeyed automatic traffic signal | 0 | 0 | 0 | 0 | |
| 302 | Disobeyed 'Give Way' or 'Stop' sign or markings | 0 | 0 | 0 | 0 | |
| 303 | Disobeyed double white lines | | 0 | 0 | 0 | |
| 304 | Disobeyed pedestrian crossing facility | 0 | 0 | 0 | 0 | |
| 305 | Illegal turn or direction of travel | 0 | 0 | 1 | 1 | 34 |
| 306 | Exceeding speed limit | 0 | 0 | 6 | 6 | 16 |
| 307 | Travelling too fast for conditions | 0 | 5 | 44 | 49 | 7 |
| 308 | Following too close | 0 | 2 | 72 | 74 | 3 |
| 309 | Vehicle travelling along pavement | 0 | 0 | 0 | 0 | |
| 310 | Cyclist entering road from pavement | 0 | 0 | 0 | 0 | |
| Driver | rider error | 8 | 41 | 374 | 423 | |
| 401 | Junction overshoot | 0 | 1 | 0 | 1 | 34 |
| 402 | Junction restart (moving off at junction) | 0 | 0 | 2 | 2 | 28 |
| 403 | Poor turn or manoeuvre | 2 | 4 | 37 | 43 | 8 |
| 404 | Failed to signal or misleading signal | 0 | 0 | 4 | 4 | 21 |
| 405 | Failed to look properly | 2 | 11 | 128 | 141 | 1 |
| 406 | Failed to judge other person's path or speed | 1 | 5 | 79 | 85 | 2 |
| 407 | pedestrian | 0 | 0 | 1 | 1 | 34 |
| 408 | Sudden braking | 1 | 5 | 57 | 63 | 4 |
| 409 | Swerved | 0 | 4 | 24 | 28 | 9 |
| 410 | Loss of control | 2 | 11 | 42 | 55 | 5 |
| Impair | ment or distraction | 0 | 7 | 37 | 44 | |
| 501 | Impaired by alcohol | 0 | 3 | 10 | 13 | 12 |
| 502 | Impaired by drugs (illicit or medicinal) | 0 | 0 | 0 | 0 | 10 |
| 503 | | 0 | 3 | 9 | 12 | 13 |
| 504 | Uncorrected, detective eye sight | 0 | 0 | 0 | 0 | <u></u> |
| 505 | liness or disability, mental or physical | 0 | 0 | 4 | 4 | 21 |
| 506 | Not displaying lights at hight or in poor visibility | 0 | 0 | 0 | 0 | |
| 507 | Cyclist wearing dark clothing at hight | 0 | 0 | 0 | 0 | |

| 508 | Driver using mobile phone | 0 | 0 | 0 | 0 | |
|--------|--|---|---|----|----|----|
| 509 | Distraction in vehicle | 0 | 1 | 9 | 10 | 14 |
| 510 | Distraction outside vehicle | 0 | 0 | 5 | 5 | 20 |
| Behav | iour or inexperience | 1 | 5 | 67 | 73 | |
| 601 | Aggressive driving | 0 | 0 | 6 | 6 | 16 |
| 602 | Careless, reckless or in a hurry | 1 | 4 | 48 | 53 | 6 |
| 603 | Nervous, uncertain or panic | 0 | 0 | 3 | 3 | 24 |
| 604 | Driving too slow for conditions or slow vehicle (e.g. tractor) | 0 | 1 | 0 | 1 | 34 |
| 605 | Learner or inexperienced driver / rider | 0 | 0 | 7 | 7 | 15 |
| 606 | Inexperience of driving to the left | 0 | 0 | 3 | 3 | 24 |
| 607 | Unfamiliar with model of the vehicle | 0 | 0 | 0 | 0 | |
| Vision | affected | 0 | 3 | 35 | 38 | |
| 701 | Stationary or parked vehicle(s) | 0 | 0 | 0 | 0 | |
| 702 | Vegetation | 0 | 0 | 0 | 0 | |
| 703 | Road layout (e.g. bend, winding road, hill crest) | 0 | 0 | 0 | 0 | |
| 704 | Buildings, road signs, street furniture | 0 | 0 | 0 | 0 | |
| 705 | Dazzling headlights | 0 | 0 | 1 | 1 | 34 |
| 706 | Dazzling sun | 0 | 0 | 1 | 1 | 34 |
| 707 | Rain, sleet, snow or fog | | 2 | 4 | 6 | 16 |
| 708 | Spray from other vehicles | 0 | 0 | 2 | 2 | 28 |
| 709 | Visor or windscreen dirty or scratched | 0 | 0 | 0 | 0 | |
| 710 | Vehicle blind spot | 0 | 1 | 27 | 28 | 9 |
| Pedes | trian involved | 2 | 0 | 1 | 3 | |
| 801 | Crossing road masked by stationary or parked vehicle | 0 | 0 | 0 | 0 | |
| 802 | Failed to look properly | 0 | 0 | 1 | 1 | 34 |
| 803 | Failed to judge vehicle's path or speed | 0 | 0 | 0 | 0 | |
| 804 | Wrong use of pedestrian crossing facility | 0 | 0 | 0 | 0 | |
| 805 | Dangerous action in carriageway (e.g. playing) | 1 | 0 | 0 | 1 | 34 |
| 806 | Impaired by alcohol | 0 | 0 | 0 | 0 | |
| 807 | Impaired by drugs (illicit or medicinal) | 0 | 0 | 0 | 0 | |
| 808 | Careless, reckless or in a hurry | 1 | 0 | 0 | 1 | 34 |
| 809 | Pedestrian wearing dark clothing at night | 0 | 0 | 0 | 0 | |
| 810 | Disability or illness, mental or physical | 0 | 0 | 0 | 0 | |
| Specia | al codes | 0 | 1 | 8 | 9 | |
| 901 | Stolen vehicle | 0 | 0 | 0 | 0 | |
| 902 | Vehicle in course of crime | 0 | 0 | 1 | 1 | 34 |
| 903 | Emergency vehicle on a call | 0 | 0 | 2 | 2 | 28 |
| 904 | Vehicle door opened or closed negligently | 0 | 0 | 0 | 0 | |
| 999 | Other | 0 | 1 | 5 | 6 | 16 |

Contributory factors by severity after period

| Code | Contributory factor group | Fatal | Serious | Slight | Total |
|---------|------------------------------|-------|---------|--------|-------|
| 101-109 | Road environment contributed | 0 | 0 | 18 | 18 |
| 201-206 | Vehicle defects | 0 | 1 | 7 | 8 |
| 301-310 | Injudicious action | 0 | 3 | 36 | 39 |
| 401-410 | Driver/rider error | 2 | 10 | 213 | 225 |
| 501-510 | Impairment or distraction | 0 | 4 | 16 | 20 |
| 601-607 | Behaviour or inexperience | 1 | 2 | 61 | 64 |
| 701-710 | Vision affected | 0 | 2 | 22 | 24 |
| 801-810 | Pedestrian involved | 2 | 0 | 0 | 2 |
| 901-999 | Special codes | 0 | 0 | 10 | 10 |
| Total | | 5 | 22 | 383 | 410 |



| Rank | 1 to 10 |
|------|---------|
| Rank | 11 to |
| | 15 |

| Code | Contributory factors | Fatal | Serious | Slight | Total | Rank |
|---------|--|-------|---------|--------|-------|------|
| Road | environment contributed | 0 | 0 | 18 | 18 | |
| 101 | Poor or defective road surface | 0 | 0 | 1 | 1 | 25 |
| 102 | Deposit on road (e.g. oil, mud, chippings) | 0 | 0 | 2 | 2 | 21 |
| 103 | Slippery road (due to weather) | 0 | 0 | 12 | 12 | 10 |
| 104 | Inadequate or masked signs or road markings | 0 | 0 | 0 | 0 | |
| 105 | Defective traffic signals | 0 | 0 | 1 | 1 | 25 |
| 106 | Traffic calming (e.g. speed cushions, road | 0 | 0 | 0 | 0 | |
| 107 | Temporary road layout (e.g. contraflow) | 0 | 0 | 0 | 0 | |
| 108 | Road lavout (e.g. bend. hill, narrow carriageway) | 0 | 0 | 1 | 1 | 25 |
| 109 | Animal or object in carriageway | 0 | 0 | 1 | 1 | 25 |
| Vehicl | e defects | 0 | 1 | 7 | 8 | |
| 201 | Tyres illegal, defective or under-inflated | 0 | 0 | 2 | 2 | 21 |
| 202 | Defective lights or indicators | 0 | 0 | 0 | 0 | |
| 203 | Defective brakes | 0 | 0 | 1 | 1 | 25 |
| 204 | Defective steering or suspension | 0 | 1 | 3 | 4 | 16 |
| 205 | Defective or missing mirrors | 0 | 0 | 0 | 0 | |
| 206 | Overloaded or poorly loaded vehicle or trailer | 0 | 0 | 1 | 1 | 25 |
| Injudio | cious action | 0 | 3 | 36 | 39 | |
| 301 | Disobeyed automatic traffic signal | 0 | 0 | 0 | 0 | |
| 302 | Disobeyed 'Give Way' or 'Stop' sign or markings | 0 | 0 | 0 | 0 | |
| 303 | Disobeyed double white lines | 0 | 0 | 0 | 0 | |
| 304 | Disobeyed pedestrian crossing facility | 0 | 0 | 0 | 0 | |
| 305 | Illegal turn or direction of travel | 0 | 0 | 0 | 0 | |
| 306 | Exceeding speed limit | 0 | 0 | 4 | 4 | 16 |
| 307 | Travelling too fast for conditions | 0 | 0 | 10 | 10 | 11 |
| 308 | Following too close | 0 | 3 | 22 | 25 | 6 |
| 309 | Vehicle travelling along pavement | 0 | 0 | 0 | 0 | |
| 310 | Cyclist entering road from pavement | 0 | 0 | 0 | 0 | |
| Driver | /rider error | 2 | 10 | 213 | 225 | |
| 401 | Junction overshoot | 0 | 0 | 0 | 0 | |
| 402 | Junction restart (moving off at junction) | 0 | 0 | 0 | 0 | |
| 403 | Poor turn or manoeuvre | 0 | 1 | 26 | 27 | 4 |
| 404 | Failed to signal or misleading signal | 0 | 0 | 0 | 0 | |
| 405 | Failed to look properly | 1 | 7 | 72 | 80 | 1 |
| 406 | Failed to judge other person's path or speed | 1 | 0 | 52 | 53 | 2 |
| 407 | Passing too close to cyclist, horse fider or pedestrian | 0 | 0 | 0 | 0 | |
| 408 | Sudden braking | 0 | 2 | 23 | 25 | 6 |
| 409 | Swerved | 0 | 0 | 13 | 13 | 9 |
| 410 | Loss of control | 0 | 0 | 27 | 27 | 4 |
| Impair | ment or distraction | 0 | 4 | 16 | 20 | |
| 501 | Impaired by alcohol | 0 | 1 | 1 | 2 | 21 |
| 502 | Impaired by drugs (illicit or medicinal) | 0 | 0 | 0 | 0 | |
| 503 | Fatigue | 0 | 1 | 3 | 4 | 16 |
| 504 | Uncorrected, defective eye sight | 0 | 0 | 0 | 0 | |
| 505 | Illness or disability, mental or physical | 0 | 1 | 5 | 6 | 13 |
| 506 | Not displaying lights at night or in poor visibility | 0 | 0 | 0 | 0 | |
| 507 | Cyclist wearing dark clothing at night | 0 | 0 | 0 | 0 | |

| 508 | Driver using mobile phone | 0 | 0 | 1 | 1 | 25 |
|--------|--|---|---|----|----|----|
| 509 | Distraction in vehicle | 0 | 1 | 4 | 5 | 15 |
| 510 | Distraction outside vehicle | 0 | 0 | 2 | 2 | 21 |
| Behav | iour or inexperience | 1 | 2 | 61 | 64 | |
| 601 | Aggressive driving | 0 | 1 | 5 | 6 | 13 |
| 602 | Careless, reckless or in a hurry | 1 | 1 | 46 | 48 | 3 |
| 603 | Nervous, uncertain or panic | 0 | 0 | 4 | 4 | 16 |
| 604 | Driving too slow for conditions or slow vehicle (e.g. tractor) | 0 | 0 | 1 | 1 | 25 |
| 605 | Learner or inexperienced driver / rider | 0 | 0 | 4 | 4 | 16 |
| 606 | Inexperience of driving to the left | 0 | 0 | 0 | 0 | |
| 607 | Unfamiliar with model of the vehicle | 0 | 0 | 1 | 1 | 25 |
| Vision | affected | 0 | 2 | 22 | 24 | |
| 701 | Stationary or parked vehicle(s) | 0 | 0 | 0 | 0 | |
| 702 | Vegetation | 0 | 0 | 0 | 0 | |
| 703 | Road layout (e.g. bend, winding road, hill crest) | 0 | 0 | 1 | 1 | 25 |
| 704 | Buildings, road signs, street furniture | 0 | 0 | 0 | 0 | |
| 705 | Dazzling headlights | 0 | 0 | 0 | 0 | |
| 706 | Dazzling sun | 0 | 0 | 0 | 0 | |
| 707 | 7 Rain, sleet, snow or fog | | 0 | 1 | 1 | 25 |
| 708 | 8 Spray from other vehicles | | 0 | 1 | 1 | 25 |
| 709 | Visor or windscreen dirty or scratched | 0 | 0 | 0 | 0 | |
| 710 | Vehicle blind spot | 0 | 2 | 19 | 21 | 8 |
| Pedes | trian involved | 2 | 0 | 0 | 2 | |
| 801 | Crossing road masked by stationary or parked vehicle | 0 | 0 | 0 | 0 | |
| 802 | Failed to look properly | 0 | 0 | 0 | 0 | |
| 803 | Failed to judge vehicle's path or speed | 0 | 0 | 0 | 0 | |
| 804 | Wrong use of pedestrian crossing facility | 0 | 0 | 0 | 0 | |
| 805 | Dangerous action in carriageway (e.g. playing) | 1 | 0 | 0 | 1 | 25 |
| 806 | Impaired by alcohol | 0 | 0 | 0 | 0 | |
| 807 | Impaired by drugs (illicit or medicinal) | 0 | 0 | 0 | 0 | |
| 808 | Careless, reckless or in a hurry | 1 | 0 | 0 | 1 | 25 |
| 809 | Pedestrian wearing dark clothing at night | 0 | 0 | 0 | 0 | |
| 810 | Disability or illness, mental or physical | 0 | 0 | 0 | 0 | |
| Specia | al codes | 0 | 0 | 10 | 10 | |
| 901 | Stolen vehicle | 0 | 0 | 0 | 0 | |
| 902 | Vehicle in course of crime | 0 | 0 | 0 | 0 | |
| 903 | Emergency vehicle on a call | 0 | 0 | 1 | 1 | 25 |
| 904 | Vehicle door opened or closed negligently | 0 | 0 | 0 | 0 | |
| 999 | Other | 0 | 0 | 9 | 9 | 12 |

C.2. Red X compliance

Table C-1 Summary of Red X events

| Duration (mins) | Total number of non-compliant vehicles | Per-lane average minutely flow during lane closure | Average minutely flow of non- compliant vehicles | Percentage non- compliance |
|-----------------|--|---|--|-------------------------------|
| 24 | 29 | 19 | 1 | 2% |
| 20 | 92 | 19 | 5 | 8% |
| 6 | 18 | 17 | 3 | 4% |
| 6 | 7 | 14 | 1 | 2% |
| 25 | 56 | 17 | 2 | 3% |
| 11 | 1 | 4 | 0 | 1% |
| 10 | 18 | 16 | 2 | 3% |
| 5 | 1 | 4 | 0 | 2% |
| 2 | 9 | 11 | 4 | 10% |
| 17 | 24 | 11 | 1 | 3% |
| 20 | 22 | 17 | 1 | 2% |
| 7 | 18 | 12 | 2 | 5% |
| 7 | 9 | 18 | 1 | 2% |
| 32 | 126 | 22 | 4 | 4% |
| 2 | 2 | 4 | 1 | 6% |
| 64 | 55 | 3 | 1 | 6% |
| 8 | 14 | 7 | 2 | 6% |
| 28 | 103 | 15 | 4 | 6% |
| 40 | 205 | 16 | 5 | 8% |
| 20 | 27 | 9 | 1 | 4% |
| Average: 18 | Average: 42 | Average: 13 | Average: 2 | Average: 4% |